# Memorandum

То	Sarah Schmied (Golder Associate	es Ltd.)	Page 1							
СС	Ilya Sher (AECOM), Hossein Zaro	ei (AECOM)								
Subject	Alternative Low Carbon Fuel Use at the St Marys Cement									
From	Ilia Merkoulovitch (AECOM), Aras	sh Mirhoseini (AECOM)								
Date	January 24, 2020	Project Number 606	18950							

This document is intended to present a summary of the information included in the Traffic Impact Study technical memorandum that is appended to the back of this document.

St Marys Cement (SMC) a company of Votorantim Cimentos North America (VCNA) is undertaking an initiative to use Alternative Low Carbon Fuels (ALCFs) as an energy source for their Bowmanville Cement Plant (the Site). This initiative will help reduce greenhouse gas (GHG) emissions in Ontario while diverting materials from landfills. The Site is located at 410 Bowmanville Avenue, in the Municipality of Clarington, in Durham Region, south of Highway 401.

SMC currently has an Environmental Compliance Approval to use woody materials as an ALCF at the Site and is preparing an ALCF Application under Ontario Regulation (O. Reg 79/15) of the *Environmental Protection Act* to expand the current use of ALCFs at the Site. SMC is preparing an application to support the following:

- Thermal replacement of 30% of the conventional fuels (coal, petroleum coke) used at the Site
  with ALCFs, which is an approximate increase from the current 96 tonnes of ALCFs used per
  day to 400 tonnes of ALCFs per day;
- Adding biomass, cellulosic and plastic materials derived from industrial and/or post-consumer sources, which cannot be recycled, are not considered hazardous and are not derived from animals or the processing and preparations of food, to the list of approved ALCFs at the Site based on the recent demonstration project;
- Installing new equipment at the Site to accommodate the ALCFs; and
- Increasing the capacity of the current ALCFs storage at the Site using enclosed containers and buildings.

The expanded use of ALCFs at the Site is anticipated to result in an increase of up to 35 two-way trips per day by heavy vehicles in and out the Site throughout a typical day for deliveries of ALCFs, thus, AECOM Canada Ltd. has been retained to evaluate potential impacts of the increase in heavy vehicle volumes on traffic operations at the Highway 401 / Bowmanville Avenue interchange, the interchange adjacent to the Site and to recommend potential mitigation measures, if required. For the purpose of this traffic impact study, the AECOM project team have undertaken the following tasks:

- Collect data and assess the existing traffic conditions (2019) during the typical weekday peak periods;
- Forecast annual growth in traffic volumes at the interchange and assess future background traffic conditions (i.e., future traffic conditions in the absence of the noted increase in heavy



- vehicle volumes in / out of the Site) in 2024 (i.e., the five-year horizon) during the same peak periods;
- Assess traffic conditions under the future total scenario (i.e., the future background traffic
  volumes plus the noted increase in heavy vehicle volumes) in 2024 and compare with those
  under the future background scenario during the same peak periods; and
- Summarize the traffic assessment findings and provide mitigation measures, where necessary.

As detailed in the Traffic Impact Study technical memorandum appended to the back of this document, the assessment showed that although specific movements at the study area intersections would operate at unacceptable level of service (LOS) and / or over capacity, these noted failing operations are not the result of the additional volume of traffic that would be generated by the Site. In fact, the additional heavy vehicle volumes to be generated as a result of the expanded use of ALCFs at the Site would constitute only less than 1% of the total traffic volumes entering the study area intersections and are expected to result in no change to the LOS at the study area intersections and their individual movements; i.e., no change to the estimated LOS for all the individual movements at the study area intersections as compared to the "business-as-usual" conditions (i.e., without any additional Site-generated traffic).

Given the expected failing operations of the specific individual movements at the three stop-controlled study area intersections under both the "business-and-usual" and "with additional site traffic" scenarios and although not caused by the additional Site traffic, a set of signal warrant analyses were also performed to assess need and justification for signalization of the study area intersections. The signal warrant analyses showed that among the three study area intersections, only the intersection of Bowmanville Avenue and Energy Drive meets the justification for installation of traffic signals in both the "business-as-usual" and "with additional site traffic" conditions. The other two intersections do not meet the warrant for signalization under neither the "business-and-usual" nor "with additional site traffic" scenarios.

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СС	Ilya Sher (AECOM), Hossein Zare	ei (AECOM)	
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From	Ilia Merkoulovitch (AECOM), Aras	h Mirhoseini (AECOM)	
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- Collect data and assess the existing traffic conditions (2019) during the typical weekday peak periods;
- Forecast annual growth in traffic volumes at the interchange and assess future background traffic conditions (i.e., future traffic conditions in the absence of the noted increase in heavy vehicle volumes in / out of the Site) in 2024 (i.e., the five-year horizon) during the same peak periods;



- Assess traffic conditions under the future total scenario (i.e., the future background traffic
  volumes plus the noted increase in heavy vehicle volumes) in 2024 and compare with those
  under the future background scenario during the same peak periods; and
- Summarize the traffic assessment findings and provide mitigation measures, where necessary.

### Study Area

A study area has been selected with the intention of capturing the intersections with the largest expected impact as a result of the increase in truck volume. As per the information provided by the client the noted additional volume of heavy vehicles would originate from and be destined for Highway 401. As such, the noted increase in heavy vehicle volumes would increase the total entering traffic volumes to the following three intersections only (shown in **Figure 1**).

- Bowmanville Avenue and Highway 401 Westbound Off-Ramp (North Ramp Terminal -Unsignalized)
- Bowmanville Avenue and Energy Drive (Unsignalized)
- Energy Drive and Highway 401 Eastbound Off-Ramp (South Ramp Terminal Unsignalized)



**Figure 1: Study Area Intersections** 

# **Existing Conditions**

A recently-collected set of turning movement counts for the study area intersections were obtained from MTO. The turning movement counts were collected on Wednesday, November 21st, 2018.

An additional set of turning movement counts as well as a new set of 24-hour 7-day automatic traffic recorder (ATR) data were commissioned by the AECOM project team in order to verify the data provided by MTO; i.e., to confirm that there have been no significant changes in the study area traffic patterns and volumes since when the MTO data were collected. On behalf of AECOM, Ontario Traffic Inc. (OTI) visited the study area intersections and collected the traffic data on Tuesday, November 19<sup>th</sup>, 2019. A review of the 2018 and 2019 data showed very similar traffic patterns and overall volumes. However, the 2018 traffic volumes provided by MTO were identified to be slightly greater than those collected by OTI in 2019, and as such the 2018 traffic volumes were selected as the existing condition volumes. In addition, based on the review of the traffic data, 07:15AM – 08:15AM



and 04:15PM – 05:15PM on a typical weekday have been identified as the two critical time periods for the purpose of this traffic analysis.

The turning movement volumes at the three study area intersections in the existing conditions during the identified AM and PM peak hours are shown in **Figure 2** and **Figure 3**, respectively.

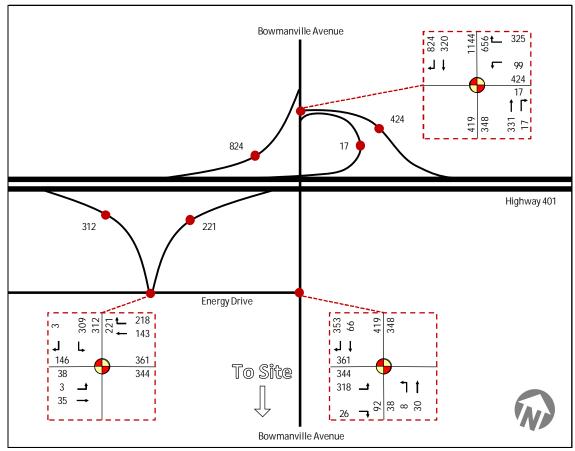


Figure 2: Existing (2019) Turning Movement Volumes - AM Peak Hour

The traffic operations at the study area intersections were analyzed using the noted traffic volumes in **Figure 2** and **Figure 3**. Synchro 9.2 software was used for the analysis, which is generally based on the methodology found in the Highway Capacity Manual (HCM). HCM 2010 methodology was selected to be used for reporting the traffic operations at each intersection.



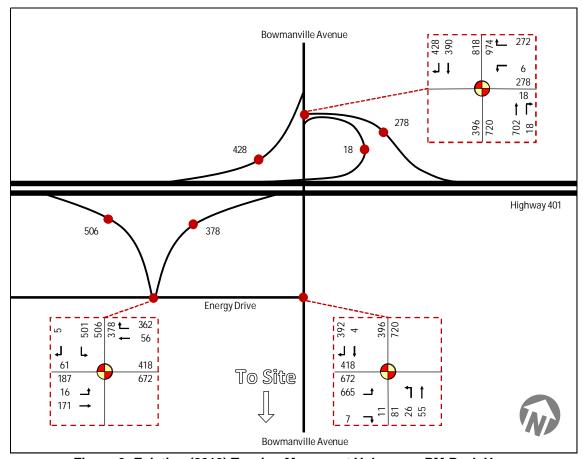


Figure 3: Existing (2019) Turning Movement Volumes – PM Peak Hour

The existing AM and PM peak hour traffic operations are analyzed using the Level of Service (LOS), volume-to-capacity ratio (V/C), control delay, and 95<sup>th</sup> percentile queue length for the subject intersections. LOS is a qualifying measure of traffic operations at an intersection, relating the delay per vehicle for a 15-minute analysis period. LOS A through D typically reflect adequate operations, while LOS E reflects increasing congestion and at capacity operations, and LOS F reflects long delays and, in some cases, severe traffic congestion. The LOS criteria for signalized and unsignalized intersection traffic controls are summarized in **Table 1**. The V/C ratio is a measure of the proportion of the calculated intersection capacity that is utilized by the actual or modeled traffic volumes. Critical movements in the table are bolded. The movements with either a V/C ratio greater than 0.85 or LOS worse than D are defined as critical. The summary of the existing conditions analysis is presented in **Table 2**. The detailed analysis outputs related to the existing conditions scenario are provided in **Appendix B**.

Table 1: Intersection LOS Criteria

Levels of Convins	Average Control Del	ay (seconds / vehicle)
Levels of Service	Traffic Signal Control	Stop Control
Α	0 to 10	0 to 10
В	>10 to 20	>10 to 15
С	>20 to 35	>15 to 25
D	>35 to 55	>25 to 35
E	>55 to 80	>35 to 50
F	>80	>50

Source: Highway Capacity Manual (2010)



			AM Pe	ak Hou	r		PM Pe	ak Hou	r
Intersection	Movement	Delay	LOS	V/C Ratio	95th % Queue (m)	Delay	LOS	V/C Ratio	95th % Queue (m)
Bowmanville Avenue	WBLR	36.4	Е	0.84	67	29.8	D	0.68	37
and Highway 401	NBTR	0.0	Α	-	-	0.0	Α	-	-
Westbound Off-Ramp	SBLT	0.0	Α	-	-	0.0	Α	-	-
	EBL	15.5	С	0.50	21	65.1	F	1.02	126
	EBR	10.1	В	0.04	1	10.1	В	0.01	0
Bowmanville Avenue	NBL	8.8	Α	0.01	0	8.2	Α	0.02	1
and Energy Drive	NBT	0.0	Α	-	-	0.0	Α	-	-
	SBT	0.0	Α	-	-	0.0	Α	-	-
	SBR	0.0	Α	-	-	0.0	Α	-	-
	EBTL	8.0	Α	0.01	0	8.2	Α	0.01	0
E	EBT	0.0	Α	-	-	0.1	Α	-	-
Energy Drive and	WBT	0.0	Α	-	-	0.0	Α	-	-
Highway 401 Eastbound Off-Ramp	WBR	0.0	Α	-	-	0.0	Α	-	-
Lasibound On-Namp	SBL	15.2	С	0.48	20	34.8	D	0.84	69
	SBR	9.6	Α	0.01	0	9.5	Α	0.01	0

Table 2: Summary of Traffic Operations in the Existing (2019) Conditions

Under the existing conditions, the only two critical movements in the existing conditions are as follows:

- Bowmanville Avenue and Highway 401 Westbound Off-Ramp (North Ramp Terminal)
  - The westbound shared left / right turn movement operates at LOS E with a delay of 36.4 seconds during the AM peak hour.
- Bowmanville Avenue and Energy Drive
  - The eastbound left-turn movement operates at LOS F with a delay of 65.1 seconds and a V/C ratio of 1.02, an indication that the movement operates above capacity during the PM peak hour.

The 95<sup>th</sup> percentile queue for the eastbound left-turn movement at the intersection of Bowmanville Avenue and Energy Drive was estimated at 126 metres and shown to reach the upstream intersection (i.e., the Highway 401 south ramp terminal).

# **Future Background Conditions**

The road network and lane configurations used in the analysis of the future background conditions are the same as those used in the existing conditions analysis. As stated earlier, the horizon year of 2024 was employed for the analysis.

In order to account for the growth in the traffic volumes entering the study area intersections due to the background developments by 2024, the most recent available historical AADT volumes on the section of Highway 401 in proximity of the Bowmanville Avenue interchange area used. The historical AADT volumes were for the five-year period between 2012 and 2016. Using the noted AADT volumes and assuming a linear growth in the historical traffic volumes, the annual growth rate was estimated at an average of 1.36% per annum. The 1.36% annual growth rate was applied to the existing (2018) turning movement volumes to project the turning movement volumes in 2024 under the future background scenario. The projected turning movement volumes at the study area intersections in the future background conditions during the AM peak hour and PM peak hour are shown in **Figure 4** and **Figure 5**, respectively.



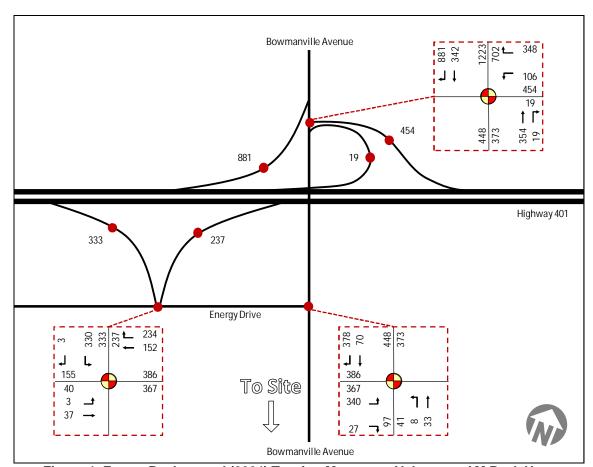


Figure 4: Future Background (2024) Turning Movement Volumes – AM Peak Hour



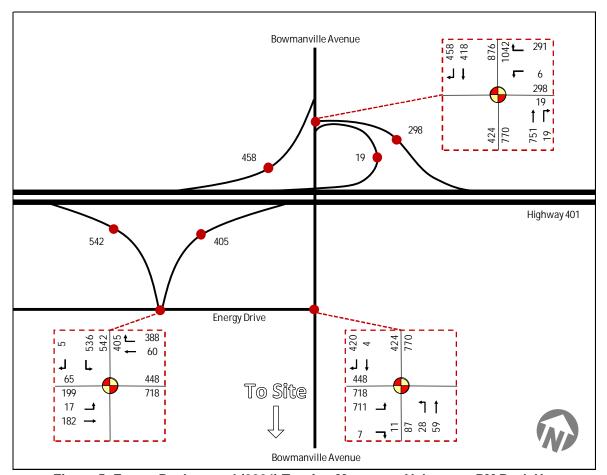


Figure 5: Future Background (2024) Turning Movement Volumes – PM Peak Hour

The future background AM and PM peak hour traffic operations are summarized in **Table 3**. The detailed analysis outputs pertaining to the future background scenario are provided in **Appendix B**.

The assessment of the future background conditions revealed generally worsened traffic operations at the study area intersections during both the AM and PM peak hours, as compared to those in the existing conditions.

During the AM peak hour, the performance of the westbound shared left / right turn movement at the north ramp terminal as the only critical movement at the three study area intersections would worsen from LOS E and V/C ratio of 0.84 in the existing conditions to LOS F and V/C ratio of 0.95 in the future background conditions. This indicates that the noted movement would be nearing capacity (i.e., a V/C ratio of 1.00 indicates the movement operates at capacity). In comparison to the existing conditions, there would be no new critical movements during the AM peak hour.

Unlike the future background conditions during the AM peak hour, in the future background conditions during the PM peak hour, there would be two (2) additional critical movements as compared to that in the existing conditions, bringing the total to three (3) critical movements. While the eastbound left-turn movement at the intersection of Bowmanville Avenue and Energy Drive would continue to operate over capacity with a V/C ratio of 1.13. The southbound left-turn movement at the South Ramp Terminal and the westbound shared left / right turn movement at the north ramp terminal are expected to reach critical conditions under the future background scenario during the PM peak hour.



and Energy Drive

Energy Drive and

Highway 401

Eastbound Off-Ramp

**PM Peak Hour** AM Peak Hour 95th % 95th % Intersection V/C V/C Movement Delav LOS Queue Delay LOS Queue Ratio Ratio (m) (m) WBLR 0.95 39.3 0.77 Bowmanville Avenue 53.9 91 49 and Highway 401 NBTR 0.0 0.0 Α Westbound Off-Ramp A **SBLT** 0.0 0.0 **EBL** 16.9 C 0.55 25 99.4 1.13 168 F **EBR** 10.2 В 10.2 В 0.01 0.04 1 0 **NBL** 1 Bowmanville Avenue 8.9 Α 0.01 0 8.3 Α 0.03

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Table 3: Summary of Traffic Operations in the Future Background (2024) Conditions

The following movements were noted as critical in the future background conditions in 2024:

Α

Α

Α

Α

A

Α

A

C

Α

- Bowmanville Avenue and Highway 401 Westbound Off-Ramp (North Ramp Terminal)
  - The westbound shared left / right turn movement would operate at LOS F with a delay of 53.9 seconds and a V/C ratio of 0.95 during the AM peak hour and at LOS E with a delay of 39.3 seconds and a V/C ratio of 0.77 during the PM peak hour.
- Bowmanville Avenue and Energy Drive

**NBT** 

SBT

**SBR** 

**EBTL** 

**EBT** 

**WBT** 

**WBR** 

SBL

SBR

0.0

0.0

0.0

8.1

0.0

0.0

0.0

16.4

9.8

- The eastbound left-turn movement would operate at LOS F with a delay of 99.4 seconds and a V/C ratio of 1.13 during the PM peak hour.
- Energy Drive and Highway 401 Eastbound Off-Ramp (South Ramp Terminal)
  - The southbound left-turn movement would operate at LOS F with a delay of 50.6 seconds and a V/C ratio of 0.94 during the PM peak hour.

The 95<sup>th</sup> percentile queue for the eastbound left-turn movement at the intersection of Bowmanville Avenue and Energy Drive was estimated at 168 metres; thus, it would reach the upstream intersection (i.e., the Highway 401 south ramp terminal).

#### **Future Total Conditions**

The road network and lane configurations used in the analysis of the future total conditions are the same as those used in the existing and future background conditions analyses. The future horizon year of 2024 was used in order to identify the impact of the increase in heavy vehicle volumes by comparing the traffic conditions in the future total scenario with those in the future background scenario.

As per the information provided by the client, the expanded use of ALCFs at the Site is anticipated to result in an increase of up to 35 two-way trips per day by heavy vehicles in and out the Site. Specific details on origins, destinations, and schedule of these additional heavy vehicle trips were not available at the time of preparation of this technical memorandum. Hence, for the purpose of this traffic analysis and as described in the following two paragraphs, the AECOM project team have made some conservative assumptions in estimating the unknown information.



The 24-hour 7-day ATR data collected at the south leg of the Bowmanville Avenue and Energy Drive intersection in November 19th, 2019 captures the temporal distribution of heavy vehicles in and out of the Site on a typical weekday in the existing (2019) conditions. For the purpose of estimating the number of additional heavy vehicle trips produced by and attracted to the Site during the AM and PM peak hours under the future total scenario, it is assumed that the temporal distribution of the additional heavy vehicle volumes (i.e., the additional 35 two-way trips per day by heavy vehicles) would be identical to those in the existing conditions. The review of the ATR data showed that the AM and PM peak hours of the heavy vehicle travel in and out of the Site are different from the previouslyidentified AM and PM peak hours for the traffic analysis which were described in the "Existing Conditions" Section of this technical memorandum. However, it has been conservatively assumed that the peak-hour volumes of the additional heavy vehicle traffic would be entering / exiting the Site during the previously-identified AM and PM peak hours for the traffic analysis. Based on the ATR data, the inbound traffic volumes to the Site during the AM peak hour and PM peak hour constitute almost 20% and 5%, respectively of the total inbound daily traffic volumes to the Site. The ATR data also show that almost 10% and 15% of the total outbound daily traffic volumes from the Site occur during the AM peak hour and PM peak hour respectively. Accordingly, it is estimated that there would be an additional seven (7) inbound and four (4) outbound heavy vehicles during the AM peak hour as well as additional two (2) inbound and six (6) outbound heavy vehicles during the PM peak hour under the future total scenario.

In addition and in order to consider the worst-case scenario regarding the distribution of additional heavy vehicle trips (i.e., with the highest level of potential impacts on traffic conditions in the study area road network), it is conservatively assumed that the additional heavy vehicle volumes would select an inbound and outbound routes to / from the Site with the highest potential impact on traffic operations at the study area intersections; i.e., it is assumed that the additional inbound heavy vehicles would make a westbound left-turn movement at the north ramp terminal and travel southbound along Bowmanville Avenue to reach the Site and the additional outbound heavy vehicles would make a northbound left-turn movement at the Bowmanville Avenue and Energy Drive intersection to access the eastbound on-ramp at the south ramp terminal.

The additional heavy vehicle volumes generated by the Site under the future total scenario during the AM and PM peak hours are shown in **Figure 6** and **Figure 7**, respectively.



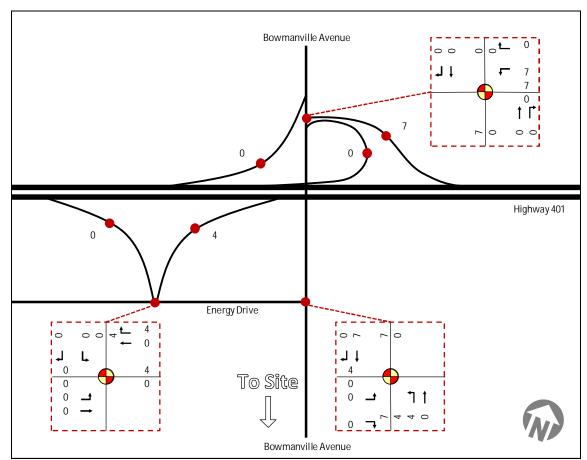


Figure 6: Additional Number of Heavy Vehicle Trips Generated by the Site in the Future Total (2024) Conditions – AM Peak Hour



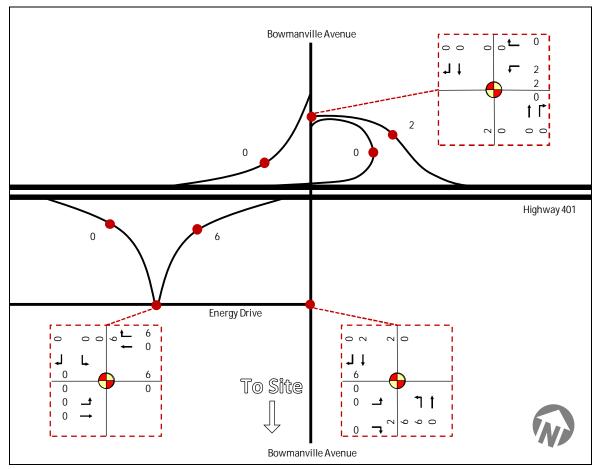


Figure 7: Additional Number of Heavy Vehicle Trips Generated by the Site in the Future Total (2024) Conditions – PM Peak Hour

The turning movement volumes at the study area intersection under the future total scenario during the AM and PM peak hours were estimated by adding the respective turning movement volumes in the future background conditions and the additional number of heavy vehicle trips generated due to the proposed expansion of the Site. The turning movement volumes under the future total scenario during the AM peak hour and PM peak hour are shown in **Figure 8** and **Figure 9**, respectively.

The future total AM and PM peak hour traffic operations are summarized in **Table 4**. The detailed analysis outputs pertaining to the future total scenario are provided in **Appendix B**.



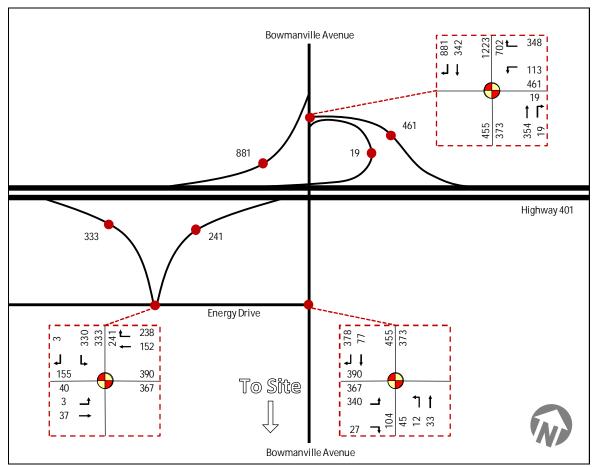


Figure 8: Future Total (2024) Turning Movement Volumes – AM Peak Hour



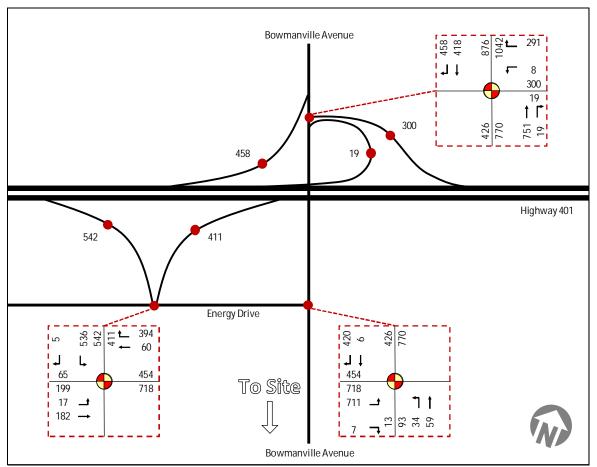


Figure 9: Future Total (2024) Turning Movement Volumes – PM Peak Hour

Table 4: Summary of Traffic Operations in the Future Total (2024) Conditions

			AM Pe	ak Hour			PM Pe	ak Hour	
Intersection	Movement	Delay	LOS	V/C Ratio	95th % Queue (m)	Delay	LOS	V/C Ratio	95th % Queue (m)
Bowmanville Avenue	WBLR	61.1	F	0.97	98	41.3	Е	0.79	50
and Highway 401	NBTR	0.0	Α	-	-	0.0	Α	-	
Westbound Off-Ramp	SBLT	0.0	Α	-	-	0.0	Α	-	
	EBL	17.7	С	0.56	26	111.9	F	1.16	179
	EBR	10.3	В	0.04	1	10.2	В	0.01	0
Bowmanville Avenue	NBL	9.3	Α	0.02	0	8.6	Α	0.03	1
and Energy Drive	NBT	0.0	Α	-	-	0.0	Α	-	-
	SBT	0.0	Α	-	-	0.0	Α	-	-
	SBR	0.0	Α	-	-	0.0	Α	-	-
	EBTL	8.1	Α	0.01	0	8.3	Α	0.02	0
Francis Debug and	EBT	0.0	Α	-	-	0.1	Α	-	-
Energy Drive and	WBT	0.0	Α	-	-	0.0	Α	-	-
Highway 401 Eastbound Off-Ramp	WBR	0.0	Α	-	-	0.0	Α	-	-
Lasibouria Off-Namp	SBL	16.5	С	0.53	23	51.3	F	0.94	94
	SBR	9.8	Α	0.01	0	9.7	Α	0.01	0



The assessment of the future total conditions showed marginal changes (if any) in the traffic conditions at the study area intersections as compared to those in the future background conditions. The LOS for the individual movements and at the overall intersection level in the future total conditions would remain unchanged from the future background conditions.

As compared to the future background conditions, there would be only marginal increases (if any) in the average vehicle delays for the individual movements in the future total conditions, with the longest increase in average vehicle delay of 12.5 seconds (equivalent to 12.6% increase) experienced by eastbound left-turning motorists at the intersection of Bowmanville Avenue and Energy Drive during the PM peak hour. However, and as described above in the "Future Background Conditions" Section, the noted movement is expected to operate under unstable traffic conditions at an unacceptable LOS F with a delay of 99.4 seconds, and over capacity with a V/C ratio of 1.13 even in the future background conditions. This explains why traffic conditions pertaining to the noted movement show a high sensitivity to even the minimal increase in traffic volumes of eight (8) additional heavy vehicles, accounting for only 0.3% of the total entering traffic volumes to the intersection of Bowmanville Avenue and Energy Drive.

During the AM peak hour, the longest increase in average vehicle delay is estimated at 7.2 seconds and to be experienced by westbound shared left / right turning motorists at the north ramp terminal.

The following movements were noted as critical in the future total conditions in 2024:

- Bowmanville Avenue and Highway 401 Westbound Off-Ramp (North Ramp Terminal)
  - The westbound shared left / right turn movement would operate at LOS F with a delay of 61.1 seconds and a V/C ratio of 0.97 during the AM peak hour and at LOS E with a delay of 41.3 seconds and a V/C ratio of 0.79 during the PM peak hour.
- Bowmanville Avenue and Energy Drive
  - The eastbound left-turn movement would operate at LOS F with a delay of 111.9 seconds and a V/C ratio of 1.16 during the PM peak hour.
- Energy Drive and Highway 401 Eastbound Off-Ramp (South Ramp Terminal)
  - The southbound left-turn movement would operate at LOS F with a delay of 51.3 seconds and a V/C ratio of 0.94 during the PM peak hour.

The 95<sup>th</sup> percentile queue for the eastbound left-turn movement at the intersection of Bowmanville Avenue and Energy Drive was estimated at 179 metres; thus, similar to the future background conditions, it would reach the upstream intersection (i.e., the Highway 401 South Ramp Terminal).

# Signal Warrant Analysis

Given the expected failing operations of the specific individual movements at the three stop-controlled study area intersections under both the future background and future total conditions and although not caused by the additional Site traffic, a set of signal warrant analyses were performed to assess needs and justification for signalization of the study area intersections. The signal warrant analyses were performed as per the guidelines provided in Ontario Traffic Manual (OTM) Book 12. As per the guidelines provided in OTM Book 12, the peak-hour traffic volumes were used to estimate the other highest hourly traffic volumes (i.e., the highest hourly traffic volumes other than the AM and PM peak hour volumes) during a typical weekday that were needed for undertaking the signal warrant analysis.

The signal warrant analyses showed that among the three study area intersections, only the intersection of Bowmanville Avenue and Energy Drive meets the justification for installation of traffic signals in the future background conditions, and thus, obviously in the future total conditions. The specific justification that is met is the Minimum Vehicular Volume justification (i.e., the OTM Justification #1). The other two intersections do not meet the warrant for signalization under neither of the future background nor future total scenarios. Note that the signal warrant analysis also showed that the north ramp terminal would meet the Minimum Four-Hour Vehicle Volume justification (i.e., the



OTM Justification #4) for signalization under the future background conditions, however, MTO does not use the Minimum Four-Hour Vehicle Volume justification (i.e., the OTM Justification #4) for assessing ramp terminals / intersections<sup>1</sup>. Signal warrant sheets are included in **Appendix C**.

# Analysis of Sensitivity to Type of Control Device at the Intersection of Bowmanville Avenue and Energy Drive

Given the signal warrant analysis findings, the traffic conditions at the Bowmanville Avenue and Energy Drive intersection were reassessed as if it would operate as a signalized intersection under both the future background and future total conditions. A standard signal phasing setup with no protected left-turn movements was used for the intersection.

**Table 5** presents the summary of traffic conditions at the intersection of Bowmanville Avenue and Energy Drive under the signalized operation in the future background conditions. Unlike the unsignalized scenario, all the individual movements under the signalized scenario would operate at an acceptable LOS D or better. The southbound right-turn movement is the only movement that would operate at a critical level during the PM peak hour with a V/C ratio of 0.89. The queue of the eastbound left-turning vehicles was estimated at 94 metres; thus, it would still extend to the upstream intersection (i.e., the South Ramp Terminal), however, the queue would be notably shorter than the 168-metre queue that would form under the unsignalized scenario.

Table 5: Summary of Traffic Conditions at the Bowmanville Avenue and Energy Drive Intersection under the Signalized Operation in the Future Background (2024) Conditions

			AM Pe	ak Hour			PM Pe	ak Hour	
Intersection	Movement	Delay	LOS	V/C Ratio	95th % Queue (m)	Delay	LOS	V/C Ratio	95th % Queue (m)
	EBL	17.1	В	0.54	51	15.9	В	0.75	94
	EBR	11.5	В	0.05	4	6.4	Α	0.01	1
Bowmanville Avenue	NBL	18.7	В	0.03	2	30.6	С	0.18	8
and Energy Drive	NBT	9.3	Α	0.07	6	15.3	В	0.11	12
	SBT	0.0	Α	0.00	19	0.0	Α	0.00	17
	SBR	17.7	В	0.67	19	41.4	D	0.89	17

**Table 6** presents the summary of traffic conditions at the intersection of Bowmanville Avenue and Energy Drive under the signalized operation in the future total conditions. similar to the future background conditions, all the individual movements under the future total scenario would operate at an acceptable LOS D or better and that the only critical movement would be the southbound right-turn movement during the PM peak hour with a V/C ratio of 0.90. The eastbound left-turn queue would be 94 metres which is the same length as that in the future background conditions; thus, it would also reach the upstream intersection (i.e., the South Ramp Terminal). Compared to the respective future background conditions, the additional heavy vehicle volumes would result in only marginal (if any) changes in traffic operations. The largest increase in delay would be 1.6 seconds and experienced by the northbound left-turning motorists during the PM peak hour.

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<sup>&</sup>lt;sup>1</sup> Ontario Traffic Manual Book 12 – Traffic Signals, March 2012, page 83.



Table 6: Summary of Traffic Conditions at the Bowmanville Avenue and Energy Drive under the Signalized Operation in the Future Total (2024) Conditions

			AM Pe	eak Hour			PM Pea	ak Hour	
Intersection	Movement	Delay	LOS	V/C Ratio	95th % Queue (m)	Delay	LOS	V/C Ratio	95th % Queue (m)
	EBL	17.1	В	0.54	51	15.9	В	0.75	94
	EBR	11.5	В	0.05	4	6.4	Α	0.01	1
Bowmanville Avenue	NBL	19.6	В	0.05	3	32.2	С	0.23	10
and Energy Drive	NBT	9.3	Α	0.07	6	15.3	В	0.11	12
	SBT	0.0	Α	0.00	20	0.0	Α	0.00	17
	SBR	18.3	В	0.69	20	42.5	D	0.90	17

Overall, the assessment of the Bowmanville Avenue and Energy Drive intersection under the signalized conditions revealed either marginal or no impact in traffic operations as a result of the increase in heavy vehicle volumes entering / exiting the Site. As the intersection would be no longer operate over-capacity, the increase in heavy vehicle volumes is expected to result in only minor incremental changes in delays and queue lengths. The detailed analysis outputs for the Bowmanville Avenue and Energy Drive intersection under the signalized operation are provided in **Appendix B**.

# **Concluding Statement**

The additional heavy vehicle volumes (estimated at up to 35 vehicles per day) to be generated as a result of the expanded use of ALCFs at the Site would constitute only less than 1% of the total traffic volumes entering the study area intersections and are expected to result in no change to the LOS at the study area intersections and their individual movements. In addition and although not driven by the additional Site traffic, the intersection of Bowmanville Avenue and Energy Drive meets the OTM justification for installation of traffic signals.

**AECOM** 

# Appendix A

**Traffic Data Collected** 

Ministry of Transportation	the Array III	Version: 1.0 Feb 1, 2016
Ministère des Transports	Intersection Layout Sheet	
2018		Work Order #
Date Variaber 1 21 Day:	Wen / Hrs: 7-9	+11 - 14 + 15 - 18
Location: MWY 401 & Wave	rly Rd 1C-431 North Ramps	Ramps: /// /
Reg/Mun:	Town/City: BOW man ville A	rea:
Observer: Aleng Mari 45kg	e: Gretch / Jamar Unit # 16 / Weather: Goudy / Goudy Re	
LHRS & O/S: 44577 0.00	Comments:	
0 21 -	<u>Schimence.</u>	
GPS: 6-Sfar 12 Datum: WGS 84 (Y) I N Lat: 43.285832 Long: -78.680066		
SIGNALIZED Y /(N) 60		4
If intersection is unsignalized; Sign Type: Stop / Yield Sign Size: 60 cm x 60 cm		
Sign	1 / 1 / 11   1   1   1   1   1   1   1	
NA: New / Good / Poor/ Missing	5 9/11/	INDICATE LOCATION &
SA: New / Good / Poor/ Missing WA: New / Good / Poor/ Missing		DIRECTION OF VEHICLE
EA: New (Good) Poor/ Missing	2 6 1 1 1 1	Vehicle NS E W
Photograph all approach's including all Signs Y / N	1	Hwy / Street Name
		HWY401 Ramp 50 (km/hr)
		the second control of
36		000000
		61
		Emergence (1994)
		The second control of the control of
Энала и пенен инчести		
50 HWY 401 Ramp		
Unit / Obrast Marra		(Sign)
Note: Show all lanes approaching and		Layout of "Special Condition"
leaving the intersection.	ime	2
Show all channelization	Hwy / Street Name	2
If there are two or more through	w//8	2
lane in one direction, indicate	主	3
if these lanes are not continuous		50
Show pedestrian crosswalks		(km/hr) Page 1 / 1



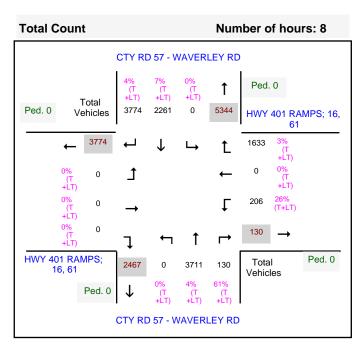
#### **TVIS II - Traffic Volume Information System**

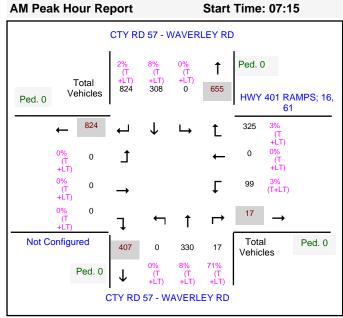
#### **Turning Movement Total Count and Peak Summary Report**

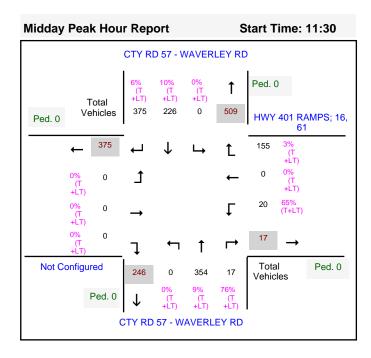
Ministry of Transportation

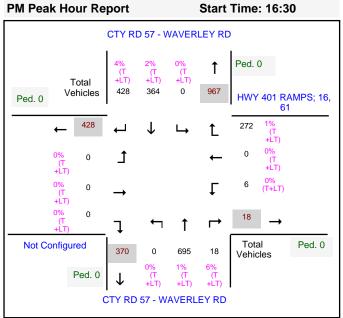
Description: HWY 401 @ WAVERLEY RD IC 431 (NRT)

Region: CENTRAL Survey Type: TM – Interchange Hwy: 401
Start Date: 21-Nov-2018 (Wed) I/C Side: N LHRS: 47577
End Date: 21-Nov-2018 (Wed) Int. Type: T - E Offset: 0











#### **TVIS II - Traffic Volume Information System**

**Turning Movement 15 Minute Report** 

Description: HWY 401 @ WAVERLEY RD IC 431 (NRT)

Region: CENTRAL Survey Type: TM – Interchange Hwy: 401
Start Date: 21-Nov-2018 (Wed) I/C Side: N LHRS: 47577

End Date: 21-Nov-2018 (Wed) Int. Type: T - E Offset: 0

								Majo	or Ro	oad	Аррі	oacl	nes															Min	or Ro	ad A	Approache	s					
					Nort	th									Sout	h									Eas	t							Not Confi	gur	ed		l
		(	CTY F	RD 57	- W	٩VE	RLE'	Y RD	)			C	TY R	D 57	- WA	VEF	RLEY	/ RD				HW	401	RAN	IPS:	Ran	np(s)	: 16	, 61								l
Start		Cars	5	Т	rucks		Lon	g Tru	ıcks			Cars		Т	rucks		Long	g Tru	cks			Cars		T	rucks		Lon	g Tru	ıcks		Cars		Trucks		Heavy Trucks	$\prod$	Total
Time	<b>←</b>	1	$\rightarrow$	←	1	$\rightarrow$	<b>←</b>	1	$\rightarrow$	Ped	<b>←</b>	1	$\rightarrow$	←	1	$\rightarrow$	<b>←</b>	1	$\rightarrow$	Ped	<b>←</b>	1	$\rightarrow$	↓	1	<b>→</b>	<b>←</b>	1	$\rightarrow$	Ped	← ↑ -	<b>→</b>	← ↑	$\rightarrow$	$\leftarrow$ $\uparrow$ $\rightarrow$	Ped	Veh.
Period	1																																				
07:00	0	67	180	0	1	1	0	4	3	C	0	43	0	0	1	0	0	2	5	0	7	0	25	0	0	1	0	0	0	0							340
07:15	0	71	204	0	2	4	0	8	2	C	0	70	0	0	1	0	0	5	3	0	5	0	41	0	0	0	1	0	1	0							418
07:30	0	71	223	0	1	2	0	7	2	C	0	67	2	0	4	1	0	7	4	0	44	0	92	0	0	1	0	0	5	0							533
07:45	0	77	180	0	1	2	0	2	1	C	0	78	0	0	3	0	0	2	2	0	34	0	99	0	0	0	1	0	1	0						Ī	483
08:00	0	63	203	0	2	0	0	3	1	C	0	89	3	0	3	0	0	1	2	0	13	0	84	1	0	0	0	0	1	0						Ī	469
08:15	0	61	175	0	3	2	0	8	3	C	0	64	0	0	6	1	0	3	1	0	6	0	63	0	0	1	2	0	0	0						Ī	399
08:30	0	48	181	0	2	3	0	6	6	C	0	79	1	0	4	1	0	2	3	0	5	0	48	0	0	1	1	0	0	0						Ī	391
08:45	0	44	115	0	0	4	0	7	7	C	0	75	0	0	4	0	0	3	5	0	2	0	45	0	0	0	0	0	0	0						Ī	311
Period	2																																				
11:00	0	31	83	0	3	2	0	3	6	C	0	78	1	0	3	0	0	7	6	0	1	0	30	0	0	3	6	0	0	0							263
11:15	0	47	75	0	3	2	0	5	2	C	0	86	1	0	5	0	0	3	1	0	3	0	37	0	0	0	2	0	1	0						Ī	273
11:30	0	46	115	0	0	6	0	3	3	C	0	79	1	0	5	2	0	3	4	0	1	0	42	0	0	0	5	0	0	0						Ī	315
11:45	0	42	68	0	4	3	0	4	1	C	0	82	2	0	4	0	0	2	3	0	2	0	36	0	0	1	2	0	2	0						Ī	258
12:00	0	53	90	0	3	3	0	4	4	C	0	86	1	0	3	0	0	4	1	0	3	0	38	0	0	1	1	0	0	0						Ī	295
12:15	0	63	79	0	0	1	0	4	2	C	0	76	0	0	7	0	0	3	3	0	1	0	35	0	0	0	5	0	0	0						Ī	279
12:30	0	51	96	0	4	5	0	4	1	C	0	89	0	0	2	0	0	4	4	0	1	0	30	0	0	1	1	0	0	0						Ì	293
12:45	0	57	78	0	1	1	0	0	1	C	0	87	2	0	0	0	0	1	2	0	2	0	33	0	0	0	3	0	0	0						Ì	268
13:00	0	70	74	0	1	0	0	2	2	(	0	75	3	0	2	0	0	4	3	0	0	0	30	0	0	1	0	0	1	0						İ	268
13:15	0	54	91	0	1	1	0	4	5	(	0	73	0	0	4	1	0	2	5	0	0	0	33	0	0	3	2	0	2	0						Ì	281
13:30	0	49	90	0	2	4	0	0	3	(	0	78	0	0	1	1	0	4	2	0	2	0	32	0	0	0	5	0	1	0						ŀ	274
13:45	0	56	91	0	0	2	0	2	4	(	0	88	3	0	1	0	0	5	3	0	1	0	27	0	0	1	5	0	1	0						ŀ	290
Period	3																																				
15:00	0	80	102	0	2	3	0	0	2	C	0	143	3	0	1	0	0	5	1	0	0	0	40	0	0	0	0	0	1	0							383
15:15	0	61	84	0	2	0	0	4	4	(	0	161	2	0	1	0	0	5	1	0	2	0	57	0	0	2	3	0	1	0						ļ	390



#### **TVIS II - Traffic Volume Information System**

**Turning Movement 15 Minute Report** 

Description: HWY 401 @ WAVERLEY RD IC 431 (NRT)

Region: CENTRAL Survey Type: TM – Interchange

Hwy: 401

End Date: 21-Nov-2018 (Wed) Int. Type: T - E Offset: 0

								Ма	jor R	oac	App	oroac	hes															Min	or R	oad	Approa	aches					
					Nort	h									Sou	th									Eas	st							No	t Configu	ıred		
		C	TY F	RD 57	- WA	¥VΕ	RLE	Y R	D			C	TY F	RD 57	- WA	VEF	RLEY	' RD				HWY	<b>40</b> 1	RA	MPS:	Rar	np(s	): 16	, 61								ĺ
Start		Cars		Т	rucks		Lor	ıg T	rucks			Cars		Т	rucks		Lon	g Tru	cks			Cars		T	rucks	5	Lon	g Tr	ucks		Ca	ars		Trucks	Heavy Trucks	s	Total
Time	←	1	$\rightarrow$	←	1	$\rightarrow$	←	1	$\rightarrow$	Ped	←	1	$\rightarrow$	←	1	$\rightarrow$	←	1	$\rightarrow$	Ped	<b>←</b>	1	$\rightarrow$	←	1	<b>→</b>	←	1	$\rightarrow$	Ped	← 1	↑ →	←	$\uparrow$ $\rightarrow$	$\leftarrow$ $\uparrow$ $\rightarrow$	Ped	Veh.
15:30	0	58	87	0	3	0	0		2	3 (	) (	170	1	0	0	1	0	5	1	0	1	0	45	1	0	1	3	(	1	0						_	383
15:45	0	89	95	0	0	3	0		4	1 (		165	0	0	1	0	0	3	1	0	2	0	54	0	0	1	1	(	1	0							421
16:00	0	89	77	0	3	2	0		4	7 (	) (	166	2	0	3	0	0	2	0	0	2	0	58	1	0	0	1	(	1	0							418
16:15	0	100	92	0	1	1	0		4	4 (	) (	169	3	0	0	0	0	1	4	0	1	0	48	0	0	0	0	C	2	0							430
16:30	0	96	101	0	0	1	0		2	3 (		160	4	0	2	0	0	1	0	0	3	0	57	0	0	0	0	C	1	0							436
16:45	0	84	95	0	0	1	0		0 ;	3 (		167	9	0	1	0	0	1	1	0	1	0	72	0	0	0	0	C	1	0							436
17:00	0	98	104	0	1	2	0		1	1 (		180	2	0	1	0	0	0	0	0	2	0	61	0	0	1	0	(	0	0							454
17:15	0	80	109	0	0	2	0		2	1 (		181	2	0	1	0	0	0	0	0	0	0	78	0	0	1	0	C	0	0							457
17:30	0	93	98	0	1	1	0		0 :	3 (		167	0	0	0	0	0	1	0	0	4	0	55	0	0	0	0	C	0	0							423
17:45	0	61	75	0	1	2	0		0 :	2 (		175	3	0	0	0	0	0	0	0	2	0	60	0	0	2	0	(	0	0							383

Ministry of Transportation	Version: 1.0 Feb 1, 2016
Ministère des Transports 2018  Intersection Layout Shee	Contract # <u>9015-E-0009</u> Work Order # 160
Date: November 21 Day: Wen / Hrs: 7 - 5	+ 11-14+15-18
Location: HWY 401 & Waverly Rd 1C-131 South Ram	IPS Ramps: SRT/
20	r.
File Name: 2475770000 Device: Gretch / Jamar Unit # 16 /	Interval 1: AM NN / PM
Observer: Alena Mariyskaya Weather: Cloudy Claudy	Road Condition: Covered / Wet
LHRS & O/S: 44577 0.00 Comments:	
GPS: G- StarIE	
Datum: WGS 84 (Y) N  Lat: 43,89 1819  Long: -78.689986	*
SIGNALIZED Y / (N) If intersection is unsignalized; (Km/hr)	<u> </u>
Sign Type: Stop / Yield	(N)
Sign Size: 60 cm x60 cm Sign Condition: NA: New / Good / Poor/ Missing SA: New / Good / Poor/ Missing WA: New / Good / Poor/ Missing	
NA: New / Good Poor/ Missing	
SA: New / Good / Poor/ Missing WA: New / Good / Poor/ Missing EA: New / Good / Poor/ Missing Photograph all approach's including all Signs Y / N	INDICATE LOCATION & DIRECTION OF VEHICLE
EA: New / Good / Poor/ Missing Photograph all approach's	Vehicle NSEW
including all Signs (V) / N (sign)	Hwy / Street Name
Stop	Service Rd (KMM)
	OB 30
	0000
	water and the second se
9309	The second section of the second seco
Energy Dr	G21
Berrice Rd )	
Note: Hwy / Street Name	Layout of "Special Condition"
Show all lanes approaching and leaving the intersection.	
Show all channelization  If there are two or more through lane in one direction, indicate	
If there are two or more through	
if these lanes are not continuous	
Show pedestrian crosswalks	(km/hr) Page 1 / 1



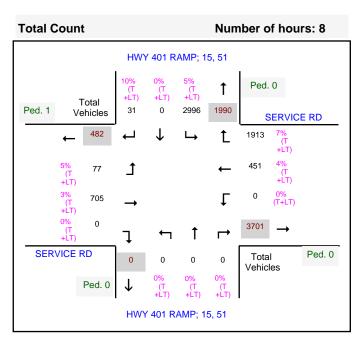
#### **TVIS II - Traffic Volume Information System**

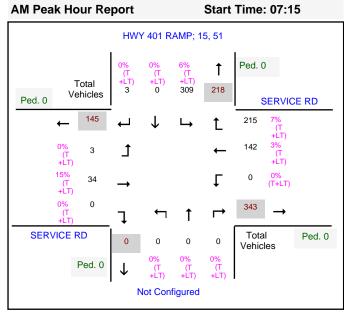
#### **Turning Movement Total Count and Peak Summary Report**

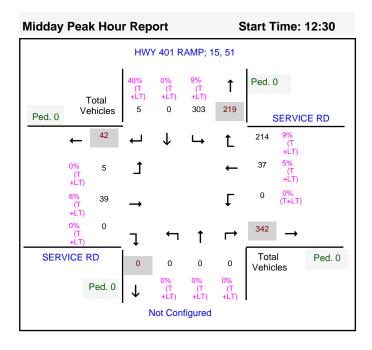
Ministry of Transportation

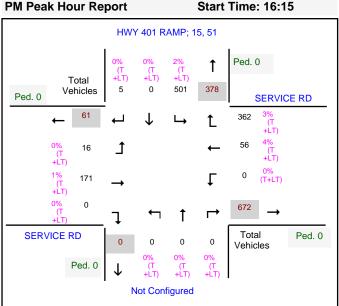
Description: HWY 401 @ WAVERLEY RD IC 431 (SRT)

Region: CENTRAL Survey Type: TM – Interchange Hwy: 401
Start Date: 21-Nov-2018 (Wed) I/C Side: S LHRS: 47577
End Date: 21-Nov-2018 (Wed) Int. Type: T - N Offset: 0











TVIS II - Traffic Volume Information System

**Turning Movement 15 Minute Report** 

Description: HWY 401 @ WAVERLEY RD IC 431 (SRT)

Start Date: 21-Nov-2018 (Wed)

Region: CENTRAL Survey Type: TM – Interchange

End Date: 21-Nov-2018 (Wed) Int. Type: T - N Offset: 0

I/C Side: S

Hwy: 401

LHRS: 47577

211d 2dio: 21 1101 2010 (110d)

								Majo	or Ro	ad	Appr	oach	nes															Mino	or Ro	ad	Approach	es								
					Eas	t									Wes	t									Nort	h							No	t Con	figu	red				
				SE	RVIC	E R	D							SE	RVIC	E RI	)					HW	<b>/</b> 40	1 RA	MP: F	Ram	p(s):	<b>15</b> ,	51											
Start		Cars		Т	rucks		Lon	g Tru	ıcks			Cars		Т	rucks		Long	g Tru	cks			Cars		Т	rucks		Lon	g Tru	cks		Cars			Truck	s	Hea	vy Tr	ucks		Total
Time	←	1	$\rightarrow$	<b>←</b>	1	$\rightarrow$	←	1	$\rightarrow$	Ped	<b>←</b>	1	$\rightarrow$	←	1	$\rightarrow$	←	1	$\rightarrow$	Ped	<b>←</b>	1	$\rightarrow$	←	1	$\rightarrow$	<b>←</b>	1	$\rightarrow$	Ped	← ↑	$\rightarrow$	<b>←</b>	1	$\rightarrow$	<b>←</b>	1	$\rightarrow$	Ped	Veh.
Period	1																																							
07:00	0	26	39	0	0	0	0	0	3	0	0	9	0	0	0	0	0	0	0	0	35	0	0	2	0	0	1	0	0	0									L	115
07:15	0	29	49	0	2	1	0	0	1	0	0	9	0	0	0	0	0	0	0	0	66	0	2	1	0	0	4	0	0	0									L	164
07:30	0	43	43	0	0	1	0	0	3	0	2	6	0	0	0	0	0	0	0	0	67	0	0	5	0	0	3	0	0	0										173
07:45	0	42	51	0	0	0	0	1	3	0	0	5	0	0	2	0	0	0	0	0	79	0	0	1	0	0	2	0	0	0										186
08:00	0	24	56	0	1	1	0	0	6	0	1	9	0	0	2	0	0	1	0	0	78	0	1	1	0	0	2	0	0	0										183
08:15	0	8	55	0	1	1	0	0	2	0	0	4	0	0	1	0	0	0	0	0	64	0	0	3	0	0	7	0	0	0										146
08:30	0	11	41	0	1	2	0	0	1	0	1	9	0	1	0	0	0	0	0	0	69	0	1	6	0	0	3	0	0	0									Γ	146
08:45	0	10	40	0	0	0	0	0	4	0	3	13	0	0	1	0	0	0	0	0	61	0	0	2	0	0	7	0	0	0										141
Period 2	2																																							
11:00	0	4	30	0	1	1	0	0	3	0	0	16	0	0	1	0	0	0	0	0	63	0	0	3	0	0	2	0	0	0										124
11:15	0	14	34	0	0	2	0	1	8	0	4	26	0	0	0	0	0	0	0	0	65	0	2	4	0	0	4	0	0	0										164
11:30	0	17	28	0	0	1	0	0	4	0	1	11	0	0	1	0	0	1	0	1	66	0	0	3	0	0	2	0	0	0										135
11:45	0	13	34	0	1	2	0	0	9	0	2	18	0	0	1	0	0	0	0	0	58	0	2	3	0	0	4	0	0	0									Г	147
12:00	0	10	45	0	1	2	0	0	1	0	3	14	0	1	0	0	0	1	0	0	66	0	0	1	0	0	3	0	0	0										148
12:15	0	11	43	0	0	0	0	0	1	0	1	12	0	0	3	0	0	0	0	0	61	0	0	3	0	0	4	0	0	0										139
12:30	0	8	45	0	1	3	0	1	2	0	2	10	0	0	1	0	0	0	0	0	74	0	2	2	0	0	5	0	0	0										156
12:45	0	10	50	0	0	0	0	0	4	0	1	13	0	0	0	0	0	0	0	0	69	0	1	0	0	0	4	0	0	0										152
13:00	0	8	57	0	0	1	0	0	5	0	0	7	0	0	1	0	0	0	0	0	67	0	0	1	0	1	7	0	0	0										155
13:15	0	9	42	0	0	2	0	0	3	0	2	6	0	0	1	0	0	0	0	0	66	0	0	2	0	1	6	0	0	0										140
13:30	0	8	37	0	0	2	0	0	3	0	3	6	0	0	1	0	1	0	0	0	79	0	0	1	0	0	3	0	0	0										144
13:45	0	8	46	0	0	0	0	0	3	0	2	12	0	0	0	0	0	0	0	0	78	0	0	0	0	0	6	0	0	0										155
Period :	3																																							
15:00	0	5	78	0	0	1	0	0	5	0	1	26	0	0	0	0	0	0	0	0	115	0	2	1	0	0	5	0	0	0										239
15:15	0	3	54	0	1	0	0	0	5	0	6	38	0	0	0	0	0	0	0	0	125	0	1	1	0	0	5	0	1	0										240



TVIS II - Traffic Volume Information System

**Turning Movement 15 Minute Report** 

Description: HWY 401 @ WAVERLEY RD IC 431 (SRT)

Region: CENTRAL Survey Type: TM – Interchange Hwy: 401

End Date: 21-Nov-2018 (Wed) Int. Type: T - N Offset: 0

								M	lajo	r Ro	ad /	Appr	oach	nes															Mi	nor	Roa	d A	pproa	che	S							
					Eas	st										Wes	st									Nor	th								ı	Not	Confi	gure	ed			1
				SE	RVIC	E R	D								SE	RVIC	E RI	)					HW	<b>/</b> 40	1 RA	MP:	Ran	np(s	: 15	5, 51		ı										1
Start		Cars		Т	rucks	5	Lo	ng	Truc	ks			Cars		Т	rucks		Long	g Tru	cks			Cars		T	ruck	s	Lo	ng T	ruck	s		Ca	rs		T	rucks		Heavy	y Truck	is	Tota
Time	←	1	$\rightarrow$	←	1	$\rightarrow$	←		1	$\rightarrow$	Ped	←	1	$\rightarrow$	←	1	$\rightarrow$	←	1	$\rightarrow$	Ped	←	1	<b>→</b>	<b>←</b>	1	$\rightarrow$	←	1	-	÷   5		← ↑	٠.	<b>→</b>	←	<b>↑</b> -	→	←	<b>1</b> -	Ped	Veh
15:30	0	6	61	0	1	2	Ī	0	1	6	0	7	48	0	0	1	0	1	1	0	0	112	0	2	0	0	0	4		0	0	0			•			•			_	25
15:45	0	10	70	0	0	0		0	1	3	0	4	36	0	0	0	0	0	0	0	0	134	0	6	2	0	0	1		0	0	0										26
16:00	0	10	86	0	0	2		0	2	3	0	3	39	0	0	0	0	0	0	0	0	124	0	0	1	0	0	2		0	0	0										27
16:15	0	16	88	0	2	2		0	0	2	0	4	35	0	0	0	0	0	0	0	0	133	0	1	0	0	0	1		0	0	0										284
16:30	0	18	101	0	0	0		0	0	3	0	3	33	0	0	0	0	0	0	0	0	116	0	2	1	0	0	2		0	0	0										279
16:45	0	11	76	0	0	0		0	0	1	0	4	45	0	0	0	0	0	0	0	0	128	0	1	1	0	0	1		0	0	0										268
17:00	0	9	86	0	0	2		0	0	1	0	5	57	0	0	1	0	0	0	0	0	116	0	1	1	0	0	1		0	0	0										280
17:15	0	5	83	0	0	0		0	0	2	0	3	43	0	0	0	0	0	0	0	0	136	0	0	2	0	0	C		0	0	0										274
17:30	0	18	71	0	0	1		0	0	2	0	2	38	0	0	0	0	0	0	0	0	126	0	1	0	0	0	2		0	0	0										26
17:45	0	7	58	0	0	2		0	0	0	0	3	30	0	0	0	0	0	0	0	0	141	0	0	1	0	0	1		0	0	0										243

Ministry of Transportation	Version: 1.0 Feb 1, 2016
	n Layout Sheet Contract # 9015-E-0009
2018	Work Order # 161
Date: November 21 Day: Wen /	Hrs: 7 - 5 + 11 - 14 + 15 - 18
Location: HWY401 & Service Rd/Waver	ley Rd (Energy Dr) Ramps:
Reg/Mun: CR Town/City: Bow	manville Area:
File Name: <u>0415770000</u> Device: Gretch / Jamar	Unit # Interval 1: AM/ NN / PM
Observer: Alena Mariyskaya Weather:	Cloudy / Cloudy Road Condition: Covered / Wet
LHRS & O/S: 47577 0.00 Comments:	
GPS: G-Stour IV	
Datum: WGS 84 (V) N Lat: 43, 8932 78	
Long: -78,688813	
SIGNALIZED Y / (N) If intersection is unsignalized;	
Sign Type: (Stop)/ Yield	
Sign Size: 60 cm x 60 cm	
Sign Condition:	
SA: New / Good / Poor/ Missing	INDICATE LOCATION &
WA: New / Good / Poor/ Missing £ 3 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	DIRECTION OF VEHICLE
Photograph all approach's	Vehicle NSEW
including all Signs (Y)/ N (sign)	Hwy / Street Name
	(km/hr)
	The same and the s
The state of the s	
9000	The second secon
	O Commence and the comm
Energy Dr	
General Service Rel > Stop	(Sign)
Note: Hwy / Street Name Show all lanes approaching and	Layout of "Special Condition"
leaving the intersection.	8 8 6
Show all channelization	Hwy/Street Name
If there are two or more through	1/Stre
If there are two or more through lane in one direction, indicate	HWY/S Vavea
if these lanes are not continuous	
Show pedestrian crosswalks	50 (km/hr) Page 1 / 1



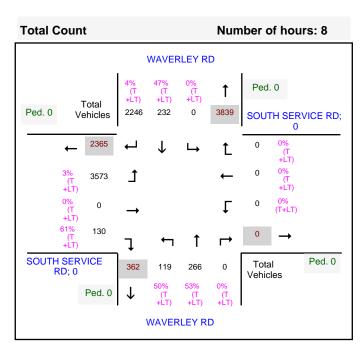
#### TVIS II - Traffic Volume Information System

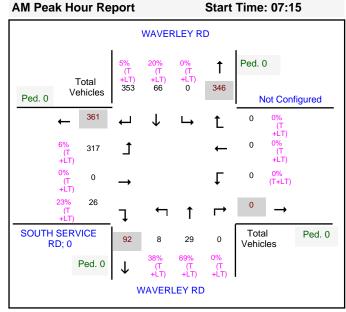
#### **Turning Movement Total Count and Peak Summary Report**

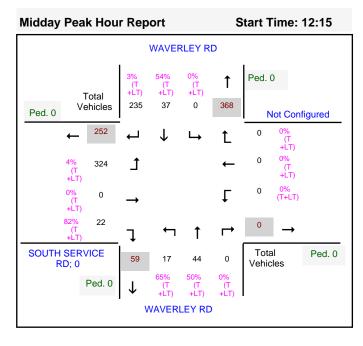
Ministry of Transportation

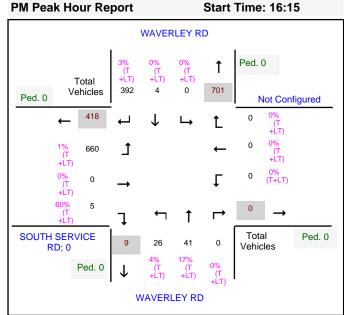
Description: HWY 401 - SERVICE RD/WAVERLEY RD

Region: CENTRAL Survey Type: TM – Interchange Hwy: 401
Start Date: 21-Nov-2018 (Wed) I/C Side: S LHRS: 47577
End Date: 21-Nov-2018 (Wed) Int. Type: T - W Offset: 0.100











**TVIS II - Traffic Volume Information System** 

**Turning Movement 15 Minute Report** 

Description: HWY 401 - SERVICE RD/WAVERLEY RD

Region: CENTRAL Survey Type: TM – Interchange Hwy: 401

 Start Date: 21-Nov-2018 (Wed)
 I/C Side: S
 LHRS: 47577

 End Date: 21-Nov-2018 (Wed)
 Int. Type: T - W
 Offset: 0.100

								Majo	or Ro	ad	Appr	oach	es															Mino	r Ro	ad A	Approach	nes							
					Nort	h									Sout	h									Wes	st							No	t Conf	igur	ed			
				WAV	/ERL	EY.	RD							WA۱	/ERL	EY F	RD					SOU	TH S	SERV	/ICE	RD:	Ram	p(s)	0	- 1									
Start	(	Cars		Т	rucks		Lon	g Tru	icks		(	Cars		Ti	rucks		Lon	g Truc	cks			Cars		Ti	rucks		Lon	g Tru	cks		Cars			Trucks		Heav	y Truck	s	Total
Time	<b>←</b>	1	$\rightarrow$	<b>←</b>	1	$\rightarrow$	←	1	$\rightarrow$	Ped	<b>←</b>	1	$\rightarrow$	←	1	$\rightarrow$	←	1	$\rightarrow$	Ped	←	1	$\rightarrow$	←	<b>↑</b>	$\rightarrow$	<b>←</b>	1	$\rightarrow$	Ped	← ↑	$\rightarrow$	←	1	$\rightarrow$	<b>←</b>	↑ -	Ped	Veh.
Period	1																																						
07:00	0	8	66	0	0	0	0	4	1	0	0	0	0	0	0	0	2	5	0	0	43	0	2	1	0	0	1	0	1	0									134
07:15	0	6	75	0	0	3	0	4	1	0	1	0	0	0	0	0	0	5	0	0	70	0	4	1	0	0	3	0	1	0									174
07:30	0	26	86	0	0	1	0	5	3	0	0	2	0	0	2	0	0	6	0	0	65	0	7	4	0	1	3	0	0	0									211
07:45	0	16	92	0	0	0	0	1	4	0	0	1	0	0	1	0	0	4	0	0	78	0	7	2	0	1	0	0	2	0									209
08:00	0	5	83	0	0	2	0	3	3	0	4	6	0	0	0	0	3	2	0	0	86	0	2	3	0	0	2	0	1	0									205
08:15	0	4	59	0	1	2	0	5	2	0	0	1	0	0	2	0	0	3	0	0	65	0	3	4	0	0	0	0	7	0									158
08:30	0	5	51	0	0	2	0	8	1	0	1	1	0	1	2	0	0	3	0	0	78	0	1	4	0	2	1	0	2	0									163
08:45	0	0	48	0	0	0	0	3	3	0	1	1	0	0	1	0	1	5	0	0	71	0	1	2	0	1	4	0	3	0									145
Period :	2																																						
11:00	0	1	33	0	1	2	0	9	0	0	0	4	0	0	0	0	3	12	0	0	81	0	0	3	0	1	1	0	1	0									152
11:15	0	1	45	0	0	2	0	1	5	0	1	2	0	0	2	0	4	3	0	0	85	0	4	3	0	1	2	0	2	0									163
11:30	0	1	44	0	0	1	0	7	1	0	1	3	0	0	3	0	3	5	0	0	79	0	0	4	0	0	1	0	2	0									155
11:45	0	3	44	0	2	3	0	2	3	0	2	5	0	0	1	0	6	5	0	0	73	0	1	3	0	1	0	0	4	0									158
12:00	0	5	53	0	1	2	0	5	1	0	1	7	0	1	2	0	1	5	0	0	78	0	0	1	0	0	1	0	3	0									167
12:15	0	5	53	0	1	0	0	6	0	0	1	7	0	0	2	0	0	4	0	0	75	0	1	5	0	1	1	0	2	0									164
12:30	0	2	53	0	1	3	0	6	1	0	2	2	0	1	0	0	3	6	0	0	79	0	1	2	0	2	2	0	3	0									169
12:45	0	3	58	0	0	0	0	5	0	0	1	6	0	0	0	0	3	4	0	0	84	0	1	0	0	0	0	0	4	0									169
13:00	0	7	65	0	0	1	0	1	1	0	2	7	0	0	1	0	4	5	0	0	73	0	1	1	0	1	2	0	5	0									177
13:15	0	1	49	0	0	2	0	3	1	0	0	1	0	0	2	0	2	6	0	0	70	0	2	3	0	0	1	0	5	0									148
13:30	0	4	44	0	0	2	0	5	0	0	1	1	0	0	1	0	3	5	0	0	83	0	2	1	0	1	0	0	3	0									156
13:45	0	3	54	0	1	0	0	6	1	0	0	2	0	0	1	0	2	5	0	0	86	0	2	0	0	0	4	0	2	0									169
Period	3																																						
15:00	0	0	80	0	1	1	0	0	0	0	3	3	0	0	0	0	5	2	0	0	140	0	1	1	0	1	2	0	2	0									242
15:15	0	5	58	0	1	1	0	4	2	0	0	6	0	0	1	0	3	4	0	0	158	0	1	1	0	1	3	0	1	0									250



#### **TVIS II - Traffic Volume Information System**

**Turning Movement 15 Minute Report** 

Description: HWY 401 - SERVICE RD/WAVERLEY RD

Start Date: 21-Nov-2018 (Wed)

Region: CENTRAL Survey Type: TM – Interchange

I/C Side: S LHRS: 47577

Hwy: 401

Offset: 0.100

End Date: 21-Nov-2018 (Wed) Int. Type: T - W

									Мај	or R	oad	Арр	roacl	nes															Mi	nor l	Road	d Ap	proac	hes						
					N	ortl	h									Sou	th									We	st								No	t Confi	gur	ed		1
				W	AVE	RLI	EΥΙ	RD							WA	VERL	EY F	RD					SOL	TH:	SER	VICE	RD	Raı	np(	s): 0										
Start		Cars	;		Truc	ks		Lon	g Tr	ucks			Cars		Т	rucks		Long	g Tru	cks			Cars		T	ruck	s	Lo	ng T	ruck	s	Т	Car	S		Trucks		Heavy Trucks	3	Tota
Time	←	1	$\rightarrow$	<b>←</b>	- ↑		$\rightarrow$	←	1	$\rightarrow$	Ped	←	1	$\rightarrow$	<b>←</b>	1	$\rightarrow$	←	1	$\rightarrow$	Ped	<b>←</b>	1	$\rightarrow$	<b>←</b>	1	$\rightarrow$	<b>←</b>	1	-	→ Tea	. +	- ↑	$\rightarrow$	←	1	→	$\leftarrow$ $\uparrow$ $\rightarrow$	Ped	Veh.
15:30	0	1	64		0	1	3	0	3	: 3	0	3	4	0	0	0	0	4	1	0	0	168	0	0	1	0	0	4		0	0	0							_	260
15:45	0	1	79		0	0	0	0	C	2	0	1	3	0	0	0	0	2	4	0	0	171	0	2	1	0	1	0		0	2	0								269
16:00	0	0	91		0	0	2	0	3		0	4	6	0	0	0	0	0	1	0	0	162	0	0	2	0	0	2		0	0	0								278
16:15	0	0	105	5	0	0	3	0	C	2	2 0	1	4	0	0	1	0	0	3	0	0	166	0	0	0	0	0	1		0	0	0								286
16:30	0	1	105	5	0	0	0	0	C		3 0	14	14	0	0	1	0	0	1	0	0	149	0	1	1	0	0	1		0	1	0								292
16:45	0	1	82	2	0	0	0	0	C	(	0	7	9	0	0	1	0	1	0	0	0	168	0	0	0	0	0	1		0	0	0								270
17:00	0	2	90		0	0	1	0	C		0	3	7	0	0	0	0	0	0	0	0	172	0	1	1	0	1	0		0	1	0								280
17:15	0	0	85		0	0	0	0	C		0	4	4	0	0	1	0	1	0	0	0	179	0	0	1	0	0	0		0	0	0								276
17:30	0	1	91		0	0	2	0	C	•	0	0	2	0	0	0	0	1	0	0	0	167	0	2	0	0	0	1		0	1	0								269
17:45	0	4	67		0	0	0	0	C		0	0	3	0	0	0	0	0	0	0	0	171	0	1	0	0	0	0		0	1	0								248



# **TVIS II - Traffic Volume information System**

#### **Ramp Weekly Volume Summary**

Hwy: 401 Between: WAVERLEY RD IC-431-NEWCASTLE

TS: 332 and: HOLT RD IC-428-NEWCASTLE
Regn: CENTRAL Pattern: CR PDCS: 37

LHRS: 47577 Offset: 0 Locn: WAVERLEY RD IC-431-NEWCASTLE

Ramp: 16 Lanes: 1 Speed: Dates: 24-May-2016 to 31-May-2016

	Tue		Wed		Thu		Fri		Sat		Sun		Mon		Tue	
H. Interval	05/24		05/25	Ŗ	05/26	P	05/27	P	05/28	Ŗ	05/29	P	05/30	무	05/31	P
00:00-01:00			2		1		0		2		2		0		5	Т
01:00-02:00			2		3		3		0		1		2		2	
02:00-03:00			2		2		2		0		0		0		1	Т
03:00-04:00			1		0		4		0		0		0		0	Т
04:00-05:00			0		1		2		0		0		0		1	Т
05:00-06:00			6		4		6		3		1	4	1		5	
06:00-07:00			2		7		6		1		0		7		9	
07:00-08:00			7		6		8		3		1		12	4	12	4
08:00-09:00			12	4	16	4	12	4	5	4	0		11		12	
09:00-10:00			14		13		15		1		2		13		20	Т
10:00-11:00			18		18		11		1		0		15		11	
11:00-12:00			20	•	19	•	20	•	5		2		18	•	22	•
AM Total			86		90		89		21		9		79		100	Т
12:00-13:00	19		15		18		17		7	•	4	•	16			
13:00-14:00	20	•	12		6		17		5		1		13			
14:00-15:00	9	П	8	П	15	П	15		3	П	4		13	П		Т
15:00-16:00	11		18		13		14	4	3		0		11			
16:00-17:00	24	4	22	4	18	4	11		4		4	4	13			
17:00-18:00	11		11		9		9		2		1		15	4		
18:00-19:00	3		3		4		7		3		3		11			
19:00-20:00	4		4		4		3		7	4	4		4			
20:00-21:00	7		6		5		1		3		2		5			Т
21:00-22:00	2		4		4		1		3		3		3			
22:00-23:00	1		3		3		2		0		2		5			
23:00-00:00	2		0		4		1		1		2		3			
PM Total	113		106		103		98		41		30		112			I
24h. Total	113		192		193		187		62		39		191		100	Ī
Noon - No	on	199		196		192		119		50		109		212		

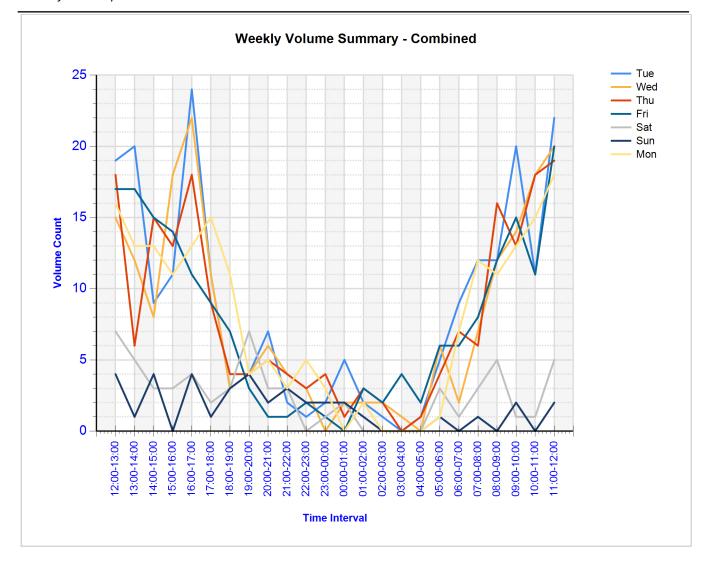
ADT	AWD
154	200



# **TVIS II - Traffic Volume information System**

Ministry of Transportation

**Ramp Weekly Volume Summary** 





# **TVIS II - Traffic Volume information System**

#### Ministry of Transportation

#### **Ramp Weekly Volume Summary**

Hwy: 401 Between: WAVERLEY RD IC-431-NEWCASTLE

TS: 332 and: HOLT RD IC-428-NEWCASTLE
Regn: CENTRAL Pattern: CR PDCS: 37

LHRS: 47577 Offset: 0 Locn: WAVERLEY RD IC-431-NEWCASTLE

Ramp: 36 Lanes: 1 Speed: Dates: 24-May-2016 to 31-May-2016

	Tue		Wed		Thu		Fri		Sat		Sun		Mon		Tue	
H. Interval	05/24	$\Box$	05/25	무	05/26	무	05/27	Ŗ	05/28	P	05/29	P	05/30	무	05/31	Ŗ
00:00-01:00			38		46		34		62		85		28		34	
01:00-02:00			25		24		26		28		45		17		19	
02:00-03:00			20		19		22		26		33		18		19	
03:00-04:00			41		35		43		29		22		36		37	
04:00-05:00		П	169		170		173		59		39		194		160	
05:00-06:00			599		617		529		137		83		587		538	
06:00-07:00			659		709		756		226		151		507		682	
07:00-08:00			797	4	679		779	4	233		136		805	4	844	4
08:00-09:00			685		713	4	665		366	4	224	4	664		697	
09:00-10:00			419		448		430		360		274		408		434	
10:00-11:00			331		384		387		391		371		335		441	
11:00-12:00			356		379		415		462	•	375		230		330	•
AM Total		П	4139		4223		4259		2379		1838		3829		4235	
12:00-13:00	370		372		360		387		403		423	•	342	•		
13:00-14:00	391	•	378	•	390	•	416	•	390		410		309			
14:00-15:00	384	П	401		446	П	408	П	354	П	309	П	320	П		П
15:00-16:00	325		289		347		355		395	4	354	4	278			
16:00-17:00	318		371	4	400	4	363		368		308		282			
17:00-18:00	349	4	292		365		364		346		281		255			
18:00-19:00	321		308		288		371	4	300		272		292	4		
19:00-20:00	240		233		251		237		236		202		163			
20:00-21:00	186		220		241		211		171		185		169			
21:00-22:00	165		184		214		185		164		146		147			
22:00-23:00	134	П	136		162		163		131		99		136			
23:00-00:00	52		71		73		96		103		47		52			
PM Total	3235		3255		3537		3556		3361		3036		2745			
24h. Total	3235		7394		7760		7815		5740		4874		6574		4235	
Noon - No	on	7374		7478		7796	5	5935	j	5199	)	6865	5	6980		

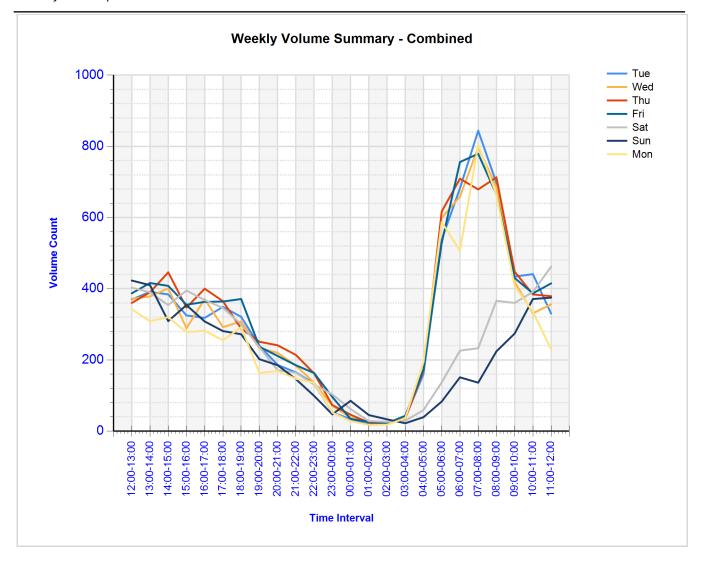
ADT	AWD
6804	7407



# **TVIS II - Traffic Volume information System**

Ministry of Transportation

**Ramp Weekly Volume Summary** 





# Project #19375 - AECOM

# **Intersection Count Report**

**Intersection:** Bowmanville Ave & Hwy 401 North Ramp Terminal

**Municipality:**Bowmanville

**Count Date:** Nov 19, 2019

**Site Code:** 1937500001

**Count Categories:** Cars, Medium Trucks, Heavy Trucks, Pedestrians

**Count Period:** 07:00-09:00, 10:00-12:00

**Weather:** Clear

### **Traffic Count Map**

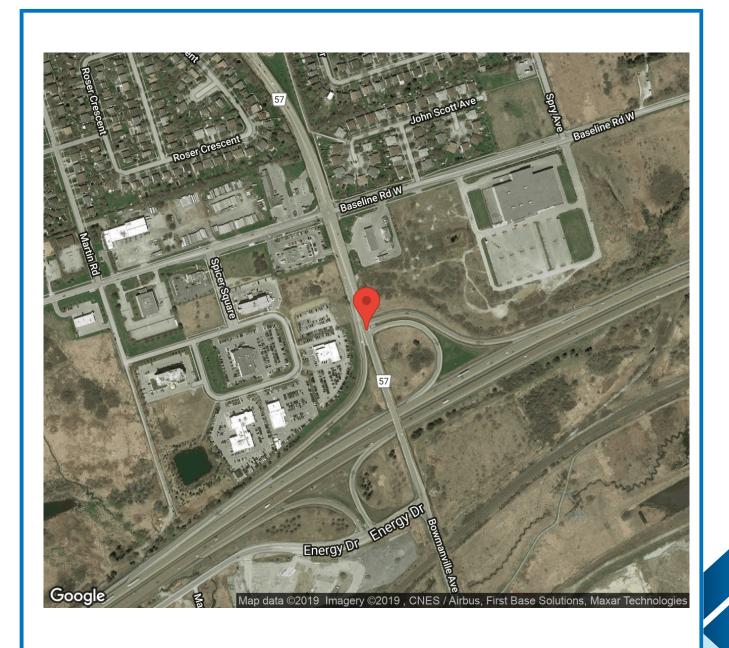


Bowmanville Ave & Hwy 401 North Ramp Intersection:

Terminal

Municipality: Bowmanville

Count Date: Nov 19, 2019



### **Traffic Count Summary**



Intersection: Bowmanville Ave & Hwy 401 North Ramp

Terminal

Municipality: Bowmanville

Count Date: Nov 19, 2019

### **Bowmanville Ave - Traffic Summary**

		North	Appr	oach 1	otals			South	Appr	oach T	otals	
	Includ	es Cars,	Mediun	ı Trucks,	Heavy Tr	ucks	Includ	es Cars,	Mediun	n Trucks,	Heavy T	rucks
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds
07:00 - 08:00	0	339	767	0	1106	0	0	342	15	0	357	0
08:00 - 09:00	0	276	692	0	968	0	0	344	14	0	358	0
					BREAK							
10:00 - 11:00	0	183	334	0	517	0	0	345	28	0	373	0
11:00 - 12:00	0	256	322	1	579	0	0	444	28	0	472	0
GRAND TOTAL	0	1054	2115	1	3170	0	0	1475	85	0	1560	0

### **Traffic Count Summary**



Intersection: Bowmanville Ave & Hwy 401 North Ramp

Terminal

Municipality: Bowmanville

Count Date: Nov 19, 2019

### **Hwy 401 North Ramp Terminal - Traffic Summary**

#### **East Approach Totals West Approach Totals Includes Cars, Medium Trucks, Heavy Trucks Includes Cars, Medium Trucks, Heavy Trucks** Hour Left Thru Right U-Turn Total Peds Left Thru Right U-Turn Total Peds 07:00 - 08:00 08:00 - 09:00 **BREAK** 10:00 - 11:00 11:00 - 12:00 **GRAND TOTAL**



Intersection: Bowmanville Ave & Hwy 401 North Ramp Terminal

Municipality: Bowmanville

Count Date: Nov 19, 2019

### North Approach - Bowmanville Ave

			Cars				Mediu	ım Truc	ks			Heav	y Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	0	38	141	0	179	0	10	0	0	10	0	12	20	0	32	0
07:15	0	60	150	0	210	0	19	0	0	19	0	16	33	0	49	0
07:30	0	60	185	0	245	0	17	1	0	18	0	17	34	0	51	0
07:45	0	68	174	0	242	0	10	1	0	11	0	12	28	0	40	0
08:00	0	35	178	0	213	0	12	0	0	12	0	18	27	0	45	0
08:15	0	50	187	0	237	0	10	1	0	11	0	16	25	0	41	0
08:30	0	40	139	0	179	0	15	3	0	18	0	17	21	0	38	0
08:45	0	37	89	0	126	0	12	1	0	13	0	14	21	0	35	0
SUBTOTAL	0	388	1243	0	1631	0	105	7	0	112	0	122	209	0	331	0



Intersection: Bowmanville Ave & Hwy 401 North Ramp Terminal

Municipality: Bowmanville
Count Date: Nov 19, 2019

### North Approach - Bowmanville Ave

			Cars				Mediu	ım Tru	cks			Heav	y Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
10:00	0	34	65	0	99	0	11	2	0	13	0	11	10	0	21	0
10:15	0	26	68	0	94	0	5	2	0	7	0	5	15	0	20	0
10:30	0	28	75	0	103	0	8	3	0	11	0	6	17	0	23	0
10:45	0	26	62	0	88	0	11	2	0	13	0	12	13	0	25	0
11:00	0	35	63	0	98	0	11	2	0	13	0	11	9	0	20	0
11:15	0	40	69	0	109	0	7	5	0	12	0	7	15	0	22	0
11:30	0	45	74	1	120	0	10	2	0	12	0	13	18	0	31	0
11:45	0	48	53	0	101	0	16	5	0	21	0	13	7	0	20	0
SUBTOTAL	0	282	529	1	812	0	79	23	0	102	0	78	104	0	182	0
GRAND TOTAL	0	670	1772	1	2443	0	184	30	0	214	0	200	313	0	513	0



Intersection: Bowmanville Ave & Hwy 401 North Ramp Terminal

Municipality: Bowmanville

Count Date: Nov 19, 2019

### **South Approach - Bowmanville Ave**

			Cars				Mediu	ım Truc	ks			Heav	y Truc	ks		
Start Time	4	1	•	J.	Total	4	1	•	J	Total	4	1	•	1	Total	Total Peds
07:00	0	31	0	0	31	0	12	0	0	12	0	13	0	0	13	0
07:15	0	55	0	0	55	0	23	0	0	23	0	25	4	0	29	0
07:30	0	51	1	0	52	0	16	0	0	16	0	17	2	0	19	0
07:45	0	62	4	0	66	0	15	0	0	15	0	22	4	0	26	0
08:00	0	61	0	0	61	0	18	0	0	18	0	24	2	0	26	0
08:15	0	55	0	0	55	0	10	0	0	10	0	8	5	0	13	0
08:30	0	55	0	0	55	0	7	0	0	7	0	12	2	0	14	0
08:45	0	66	1	0	67	0	15	0	0	15	0	13	4	0	17	0
SUBTOTAL	0	436	6	0	442	0	116	0	0	116	0	134	23	0	157	0



Intersection: Bowmanville Ave & Hwy 401 North Ramp Terminal

Municipality: Bowmanville
Count Date: Nov 19, 2019

### **South Approach - Bowmanville Ave**

		(	Cars				Mediu	ım Tru	cks			Heav	y Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
10:00	0	52	4	0	56	0	15	2	0	17	0	15	5	0	20	0
10:15	0	49	4	0	53	0	10	1	0	11	0	14	3	0	17	0
10:30	0	48	1	0	49	0	23	0	0	23	0	20	4	0	24	0
10:45	0	74	1	0	75	0	14	0	0	14	0	11	3	0	14	0
11:00	0	70	2	0	72	0	10	0	0	10	0	15	4	0	19	0
11:15	0	65	0	0	65	0	18	1	0	19	0	18	7	0	25	0
11:30	0	67	1	0	68	0	19	0	0	19	0	23	4	0	27	0
11:45	0	92	4	0	96	0	23	0	0	23	0	24	5	0	29	0
SUBTOTAL	0	517	17	0	534	0	132	4	0	136	0	140	35	0	175	0
GRAND TOTAL	0	953	23	0	976	0	248	4	0	252	0	274	58	0	332	0



Intersection: Bowmanville Ave & Hwy 401 North Ramp Terminal

Municipality: Bowmanville
Count Date: Nov 19, 2019

### **East Approach - Hwy 401 North Ramp Terminal**

			Cars				Mediu	ım Truc	cks			Heav	y Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	6	0	27	0	33	0	0	0	0	0	4	0	6	0	10	0
07:15	2	0	30	0	32	0	0	1	0	1	3	0	8	0	11	0
07:30	19	0	50	0	69	0	0	0	0	0	6	0	16	0	22	0
07:45	2	0	53	0	55	0	0	0	0	0	0	0	11	0	11	0
08:00	4	0	50	0	54	0	0	0	0	0	1	0	11	0	12	0
08:15	0	0	49	0	49	0	0	0	0	0	5	0	11	0	16	0
08:30	0	0	45	0	45	0	0	1	0	1	1	0	5	0	6	0
08:45	2	0	35	0	37	0	0	0	0	0	2	0	10	0	12	0
SUBTOTAL	35	0	339	0	374	0	0	2	0	2	22	0	78	0	100	0



Intersection: Bowmanville Ave & Hwy 401 North Ramp Terminal

Municipality: Bowmanville
Count Date: Nov 19, 2019

### **East Approach - Hwy 401 North Ramp Terminal**

			Cars				Mediu	ım Tru	cks			Heav	y Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
10:00	1	0	18	0	19	0	0	0	0	0	1	0	9	0	10	0
10:15	1	1	23	0	25	0	0	0	0	0	2	0	10	0	12	0
10:30	0	0	26	0	26	0	0	0	0	0	1	0	7	0	8	0
10:45	0	0	22	0	22	0	0	0	0	0	1	0	14	0	15	0
11:00	1	0	26	0	27	0	0	0	0	0	5	0	4	0	9	0
11:15	0	0	25	0	25	0	0	0	0	0	2	0	7	0	9	0
11:30	1	0	44	0	45	0	0	0	0	0	1	0	6	0	7	0
11:45	3	0	28	0	31	0	0	0	0	0	2	0	5	0	7	0
SUBTOTAL	7	1	212	0	220	0	0	0	0	0	15	0	62	0	77	0
GRAND TOTAL	42	1	551	0	594	0	0	2	0	2	37	0	140	0	177	0



Intersection: Bowmanville Ave & Hwy 401 North Ramp Terminal

Municipality: Bowmanville
Count Date: Nov 19, 2019

### West Approach - Hwy 401 WB On-Ramp

			Cars				Mediu	ım Tru	cks			Heav	y Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



Intersection: Bowmanville Ave & Hwy 401 North Ramp Terminal

Municipality: Bowmanville
Count Date: Nov 19, 2019

### West Approach - Hwy 401 WB On-Ramp

		(	Cars				Mediu	ım Tru	cks			Heav	y Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
10:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
GRAND TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

### **Peak Hour Diagram**

#### **Specified Period**

#### **One Hour Peak**

From: To:

07:00:00 09:00:00

07:30:00 From: To: 08:30:00

Intersection:

Bowmanville Ave & Hwy 401 North Ramp Terminal

Site ID:

1937500001

**Count Date:** 

Nov 19, 2019

Weather conditions:

#### \*\* Unsignalized Intersection \*\*

Major Road: Bowmanville Ave runs N/S

#### **North Approach**

	Out	In	Total
盘	937	431	1368
MT	52	59	111
HT	177	120	297
	1166	610	1776

#### **Bowmanville Ave**

	48	1	<b>E</b>	Ú
Totals	841	325	0	0
<b>⊟</b>	724	213	0	0
MT	3	49	0	0
HT	114	63	0	0

#### **East Approach**

	Out	In	Total
	227	5	232
MT	0	0	0
НТ	61	13	74
	288	18	306

#### **Hwy 401 WB On-Ramp**





Peds: 0

#### Peds: 0



#### **Hwy 401 North Ramp Terminal**

	Totals		MT	HT
C	0	0	0	0
Ł	251	202	0	49
-	0	0	0	0
F	37	25	0	12

#### **West Approach**

	Out	In	Total
	0	724	724
MT	0	3	3
НТ	0	114	114
	0	841	841

	4	1		J
Totals	0	359	18	0
	0	229	5	0
MT	0	59	0	0
HT	0	71	13	0

**Bowmanville Ave** 

#### **South Approach**

	Out	In	Total
	234	238	472
MT	59	49	108
ΗТ	84	75	159
	377	362	739



MT - Medium Trucks

HT - Heavy Trucks

#### **Comments**



## **Peak Hour Summary**

Intersection: Bowmanville Ave & Hwy 401 North Ramp Terminal

Count Date: Nov 19, 2019

Period: 07:00 - 09:00

### Peak Hour Data (07:30 - 08:30)

		N B	lorth A owman	pproac	:h ve			S	outh A	Approac nville A	h ve		Н	wy 40	East Ap 1 North	proacl Ramp	h Termir	ıal		Hw	West <i>i</i> y 401 \	Approac NB On-F	h Ramp		Total Vehicl
Start Time	4	1	•	J	Peds	Total	•	1	•	J	Peds	Total	4	t	•	J	Peds	Total	4	1	•	J	Peds	Total	es
07:30	0	94	220	0	0	314	0	84	3	0	0	87	25	0	66	0	0	91					0	0	492
07:45	0	90	203	0	0	293	0	99	8	0	0	107	2	0	64	0	0	66					0	0	466
08:00	0	65	205	0	0	270	0	103	2	0	0	105	5	0	61	0	0	66					0	0	441
08:15	0	76	213	0	0	289	0	73	5	0	0	78	5	0	60	0	0	65					0	0	432
Grand Total	0	325	841	0	0	1166	0	359	18	0	0	377	37	0	251	0	0	288					0	0	1831
Approach %	0	27.9	72.1	0		-	0	95.2	4.8	0		-	12.8	0	87.2	0		-						-	
Totals %	0	17.7	45.9	0		63.7	0	19.6	1	0		20.6	2	0	13.7	0		15.7						0	
PHF	0	0.86	0.96	0		0.93	0	0.87	0.56	0		0.88	0.37	0	0.95	0		0.79						0	0.93
Cars	0	213	724	0		937	0	229	5	0		234	25	0	202	0		227						0	1398
% Cars	0	65.5	86.1	0		80.4	0	63.8	27.8	0		62.1	67.6	0	80.5	0		78.8						0	76.4
Medium Trucks	0	49	3	0		52	0	59	0	0		59	0	0	0	0		0						0	111
% Medium Trucks	0	15.1	0.4	0		4.5	0	16.4	0	0		15.6	0	0	0	0		0						0	6.1
Heavy Trucks	0	63	114	0		177	0	71	13	0		84	12	0	49	0		61						0	322
% Heavy Trucks	0	19.4	13.6	0		15.2	0	19.8	72.2	0		22.3	32.4	0	19.5	0		21.2						0	17.6
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	

### **Peak Hour Diagram**

#### **Specified Period**

**One Hour Peak** 

From: To:

10:00:00 12:00:00

11:00:00 From: To: 12:00:00

Intersection: Bowmanville Ave & Hwy 401 North Ramp Terminal

Site ID: 1937500001 **Count Date:** Nov 19, 2019 Weather conditions:

#### \*\* Unsignalized Intersection \*\*

Major Road: Bowmanville Ave runs N/S

#### **North Approach**

	Out	In	Total
	428	418	846
MT	58	70	128
НТ	93	102	195
,	579	590	1169

#### **Bowmanville Ave**

	48	1	<b>L</b>	Ú
Totals	322	256	0	1
	259	168	0	1
MT	14	44	0	0
нт	49	44	0	0

#### **East Approach**

	Out	In	Total
	128	7	135
MT	0	1	1
HT	32	20	52
	160	28	188

#### **Hwy 401 WB On-Ramp**







Peds: 0



	Totals		MT	HT_
C	0	0	0	0
£	145	123	0	22
-	0	0	0	0
F	15	5	0	10

#### **West Approach**

	Out	In	Total
	0	259	259
MT	0	14	14
HT	0	49	49
	0	322	322

	4	1	•	J
Totals	0	444	28	0
	0	294	7	0
MT	0	70	1	0
HT	0	80	20	0

Peds: 0

**Bowmanville Ave** 

#### **South Approach**

	Out	In	Total
	301	173	474
MT	71	44	115
HT	100	54	154
	472	271	743



MT - Medium Trucks

HT - Heavy Trucks

#### **Comments**



## **Peak Hour Summary**

Intersection: Bowmanville Ave & Hwy 401 North Ramp Terminal

Count Date: Nov 19, 2019

Period: 10:00 - 12:00

### Peak Hour Data (11:00 - 12:00)

		l B	North A owman	pproac wille Av	h ⁄e			B	outh <i>A</i> owmai	Approac nville A	h ve		Н	wy 40	East A <sub>l</sub> 1 North	pproacl Ramp	n Termir	nal		Hw	West y 401	Approac WB On-F	h lamp		Total Vehicl
Start Time	4	1	•	J	Peds	Total	•	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	es
11:00	0	57	74	0	0	131	0	95	6	0	0	101	6	0	30	0	0	36					0	0	268
11:15	0	54	89	0	0	143	0	101	8	0	0	109	2	0	32	0	0	34					0	0	286
11:30	0	68	94	1	0	163	0	109	5	0	0	114	2	0	50	0	0	52					0	0	329
11:45	0	77	65	0	0	142	0	139	9	0	0	148	5	0	33	0	0	38					0	0	328
Grand Total	0	256	322	1	0	579	0	444	28	0	0	472	15	0	145	0	0	160					0	0	1211
Approach %	0	44.2	55.6	0.2		-	0	94.1	5.9	0		-	9.4	0	90.6	0		-						-	
Totals %	0	21.1	26.6	0.1		47.8	0	36.7	2.3	0		39	1.2	0	12	0		13.2						0	
PHF	0	0.83	0.86	0.25		0.89	0	0.8	0.78	0		0.8	0.63	0	0.73	0		0.77						0	0.92
Cars	0	168	259	1		428	0	294	7	0		301	5	0	123	0		128						0	857
% Cars	0	65.6	80.4	100		73.9	0	66.2	25	0		63.8	33.3	0	84.8	0		80						0	70.8
Medium Trucks	0	44	14	0		58	0	70	1	0		71	0	0	0	0		0						0	129
% Medium Trucks	0	17.2	4.3	0		10	0	15.8	3.6	0		15	0	0	0	0		0						0	10.7
Heavy Trucks	0	44	49	0		93	0	80	20	0		100	10	0	22	0		32						0	225
% Heavy Trucks	0	17.2	15.2	0		16.1	0	18	71.4	0		21.2	66.7	0	15.2	0		20						0	18.6
Peds % Peds					0	1					0	-					0	-					0	-	0



# Project #19375 - AECOM

### **Intersection Count Report**

**Intersection:** Bowmanville Ave & Energy Dr

**Municipality:** Bowmanville

**Count Date:** Nov 19, 2019

**Site Code:** 1937500002

**Count Categories:** Cars, Medium Trucks, Heavy Trucks, Pedestrians

**Count Period:** 07:00-09:00, 10:00-12:00

**Weather:** Clear

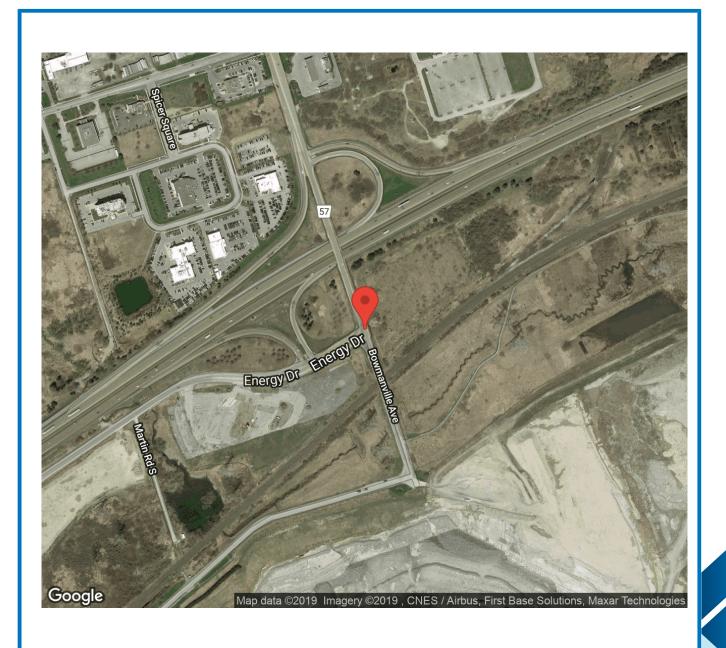


### **Traffic Count Map**

Intersection: Bowmanville Ave & Energy Dr

Municipality: Bowmanville

Count Date: Nov 19, 2019



# **Traffic Count Summary**



Intersection: Bowmanville Ave & Energy Dr

Municipality: Bowmanville
Count Date: Nov 19, 2019

### **Bowmanville Ave - Traffic Summary**

		North	Appr	oach T	otals			South	Appr	oach T	otals	
	Include	es Cars, l	Medium	Trucks,	Heavy Tr	ucks	Include	es Cars,	Medium	Trucks,	Heavy Tr	rucks
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds
07:00 - 08:00	0	96	285	0	381	0	8	44	0	0	52	0
08:00 - 09:00	0	61	230	0	291	0	13	48	0	0	61	0
					BREAK							
10:00 - 11:00	0	45	144	1	190	0	7	68	0	0	75	0
11:00 - 12:00	0	44	227	0	271	0	8	65	0	0	73	0
GRAND TOTAL	0	246	886	1	1133	0	36	225	0	0	261	0

### **Traffic Count Summary**



Intersection: Bowmanville Ave & Energy Dr

Municipality: Bowmanville Count Date: Nov 19, 2019

### **Energy Dr - Traffic Summary**

# East Approach Totals Includes Cars, Medium Trucks, Heavy Trucks Left Thru Right U-Turn Total Peds Left Thru Right U-Turn Total Peds

Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds
07:00 - 08:00	0	0	0	0	0	0	313	0	29	0	342	0
08:00 - 09:00	0	0	0	0	0	0	310	0	19	0	329	0
					BREA	<						
10:00 - 11:00	0	0	0	0	0	0	304	0	20	2	326	0
11:00 - 12:00	0	0	0	0	0	0	407	0	19	0	426	0
GRAND TOTAL	0	0	0	0	0	0	1334	0	87	2	1423	0



Intersection: Bowmanville Ave & Energy Dr

Municipality: Bowmanville
Count Date: Nov 19, 2019

### North Approach - Bowmanville Ave

			Cars				Mediu	ım Tru	cks			Heav	y Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	-	1	Total	Total Peds
07:00	0	3	41	0	44	0	3	7	0	10	0	10	6	0	16	0
07:15	0	9	53	0	62	0	4	15	0	19	0	11	8	0	19	0
07:30	0	24	55	0	79	0	9	8	0	17	0	9	14	0	23	0
07:45	0	9	61	0	70	0	2	8	0	10	0	3	9	0	12	0
08:00	0	3	36	0	39	0	0	12	0	12	0	17	2	0	19	0
08:15	0	2	48	0	50	0	1	9	0	10	0	14	7	0	21	0
08:30	0	2	38	0	40	0	1	14	0	15	0	5	13	0	18	0
08:45	0	5	34	0	39	0	1	11	0	12	0	10	6	0	16	0
SUBTOTAL	0	57	366	0	423	0	21	84	0	105	0	79	65	0	144	0



Intersection: Bowmanville Ave & Energy Dr

Municipality: Bowmanville

Count Date: Nov 19, 2019

### North Approach - Bowmanville Ave

			Cars				Mediu	ım Tru	cks			Heav	y Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
10:00	0	5	29	1	35	0	1	10	0	11	0	8	4	0	12	0
10:15	0	1	26	0	27	0	2	3	0	5	0	6	1	0	7	0
10:30	0	2	26	0	28	0	1	7	0	8	0	7	0	0	7	0
10:45	0	0	26	0	26	0	1	10	0	11	0	11	2	0	13	0
11:00	0	0	36	0	36	0	1	10	0	11	0	5	11	0	16	0
11:15	0	1	39	0	40	0	1	6	0	7	0	9	0	0	9	0
11:30	0	1	45	0	46	0	0	10	0	10	0	11	3	0	14	0
11:45	0	2	49	0	51	0	1	15	0	16	0	12	3	0	15	0
SUBTOTAL	0	12	276	1	289	0	8	71	0	79	0	69	24	0	93	0
GRAND TOTAL	0	69	642	1	712	0	29	155	0	184	0	148	89	0	237	0



Intersection: Bowmanville Ave & Energy Dr

Municipality: Bowmanville

Count Date: Nov 19, 2019

### **South Approach - Bowmanville Ave**

			Cars				Medi	ium Tru	cks			Hea	vy Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	1	1	0	0	2	1	0	0	0	1	2	0	0	0	2	0
07:15	0	1	0	0	1	0	0	0	0	0	1	21	0	0	22	0
07:30	0	1	0	0	1	0	1	0	0	1	1	1	0	0	2	0
07:45	0	1	0	0	1	0	2	0	0	2	2	15	0	0	17	0
08:00	2	2	0	0	4	3	3	0	0	6	1	12	0	0	13	0
08:15	0	1	0	0	1	0	1	0	0	1	0	9	0	0	9	0
08:30	1	0	0	0	1	0	0	0	0	0	3	5	0	0	8	0
08:45	0	0	0	0	0	0	0	0	0	0	3	15	0	0	18	0
SUBTOTAL	4	7	0	0	11	4	7	0	0	11	13	78	0	0	91	0



Intersection: Bowmanville Ave & Energy Dr

Municipality: Bowmanville

Count Date: Nov 19, 2019

### **South Approach - Bowmanville Ave**

			Cars				Medi	um Tru	cks			Hea	vy Truc	ks		
Start Time	4	1	•	J.	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
10:00	1	6	0	0	7	0	4	0	0	4	0	11	0	0	11	0
10:15	0	2	0	0	2	0	1	0	0	1	3	12	0	0	15	0
10:30	1	2	0	0	3	0	1	0	0	1	2	13	0	0	15	0
10:45	0	1	0	0	1	0	1	0	0	1	0	14	0	0	14	0
11:00	1	1	0	0	2	1	3	0	0	4	2	9	0	0	11	0
11:15	0	2	0	0	2	0	2	0	0	2	0	13	0	0	13	0
11:30	0	2	0	0	2	0	0	0	0	0	1	11	0	0	12	0
11:45	0	7	0	0	7	0	2	0	0	2	3	13	0	0	16	0
SUBTOTAL	3	23	0	0	26	1	14	0	0	15	11	96	0	0	107	0
GRAND TOTAL	7	30	0	0	37	5	21	0	0	26	24	174	0	0	198	0



Intersection: Bowmanville Ave & Energy Dr

Municipality: Bowmanville

Count Date: Nov 19, 2019

### **West Approach - Energy Dr**

			Cars				Medi	um Tru	cks			Heav	vy Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	30	0	3	0	33	12	0	0	0	12	13	0	4	0	17	0
07:15	54	0	5	0	59	23	0	1	0	24	8	0	1	0	9	0
07:30	51	0	3	0	54	15	0	0	0	15	18	0	1	0	19	0
07:45	65	0	6	0	71	13	0	2	0	15	11	0	3	0	14	0
08:00	59	0	2	0	61	15	0	0	0	15	14	0	1	0	15	0
08:15	54	0	3	0	57	9	0	1	0	10	4	0	5	0	9	0
08:30	55	0	1	0	56	7	0	0	0	7	9	0	2	0	11	0
08:45	67	0	2	0	69	15	0	0	0	15	2	0	2	0	4	0
SUBTOTAL	435	0	25	0	460	109	0	4	0	113	79	0	19	0	98	0



Intersection: Bowmanville Ave & Energy Dr

Municipality: Bowmanville
Count Date: Nov 19, 2019

### **West Approach - Energy Dr**

			Cars				Mediu	ım Tru	cks			Heav	y Truc	ks		
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
10:00	49	0	0	0	49	13	0	3	0	16	9	0	5	0	14	0
10:15	51	0	0	1	52	10	0	0	0	10	5	0	1	0	6	0
10:30	47	0	1	0	48	22	0	1	0	23	11	0	3	0	14	0
10:45	74	0	2	1	77	13	0	0	0	13	0	0	4	0	4	0
11:00	71	0	1	0	72	7	0	0	0	7	10	0	3	0	13	0
11:15	63	0	2	0	65	17	0	0	0	17	12	0	2	0	14	0
11:30	66	0	0	0	66	19	0	0	0	19	16	0	4	0	20	0
11:45	89	0	1	0	90	21	0	2	0	23	16	0	4	0	20	0
SUBTOTAL	510	0	7	2	519	122	0	6	0	128	79	0	26	0	105	0
GRAND TOTAL	945	0	32	2	979	231	0	10	0	241	158	0	45	0	203	0

# Ontario Traffic Inc.

### **Peak Hour Diagram**

**Specified Period** 

**One Hour Peak** 

From: To:

07:00:00 09:00:00 From: To: 07:15:00 08:15:00

Intersection:

Bowmanville Ave & Energy Dr

Site ID:

1937500002

Count Date: Nov 19, 2019

Weather conditions:

#### \*\* Unsignalized Intersection \*\*

Major Road: Energy Dr runs E/W

#### **North Approach**

	Out	In	Total
	250	234	484
MT	58	72	130
HT	73	100	173
	381	406	787

#### **Bowmanville Ave**

,	48	1	Ú
Totals	281	100	0
	205	45	0
MT	43	15	0
HT	33	40	0

#### **Energy Dr**

	Totals		MT	HT	
7	0	0	0	0	
4	346	229	66	51	
7	25	16	3	6	

Out

245

69

57

371

MT

HT

**West Approach** 

In

207

46

38

291

Total

452

115

95

662

Peds: 0



Peds: 0

#### 

**Bowmanville Ave** 

#### **South Approach**

	Out	In	Total
	7	61	68
MT	9	18	27
ΗТ	54	46	100
	70	125	195



MT - Medium Trucks

HT - Heavy Trucks

#### **Comments**



## **Peak Hour Summary**

Intersection: Bowmanville Ave & Energy Dr

Count Date: Nov 19, 2019

Period: 07:00 - 09:00

### **Peak Hour Data (07:15 - 08:15)**

		l B	North A owman	pproac	ch .ve			S	South <i>A</i> owma	Approac nville A	h ve				East A	pproach	1			,	West A Ener	pproacl gy Dr	h		Total Vehicl
Start Time	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	4	Peds	Total	es
07:15		24	76	0	0	100	1	22		0	0	23					0		85		7	0	0	92	215
07:30		42	77	0	0	119	1	3		0	0	4					0		84		4	0	0	88	211
07:45		14	78	0	0	92	2	18		0	0	20					0		89		11	0	0	100	212
08:00		20	50	0	0	70	6	17		0	0	23					0		88		3	0	0	91	184
Grand Total		100	281	0	0	381	10	60		0	0	70					0	0	346		25	0	0	371	822
Approach %		26.2	73.8	0		-	14.3	85.7		0		-						-	93.3		6.7	0		-	
Totals %		12.2	34.2	0		46.4	1.2	7.3		0		8.5						0	42.1		3	0		45.1	
PHF		0.6	0.9	0		0.8	0.42	0.68		0		0.76						0	0.97		0.57	0		0.93	0.96
Cars		45	205	0		250	2	5		0		7						0	229		16	0		245	502
% Cars		45	73	0		65.6	20	8.3		0		10						0	66.2		64	0		66	61.1
Medium Trucks		15	43	0		58	3	6		0		9						0	66		3	0		69	136
% Medium Trucks		15	15.3	0		15.2	30	10		0		12.9						0	19.1		12	0		18.6	16.5
Heavy Trucks		40	33	0		73	5	49		0		54						0	51		6	0		57	184
% Heavy Trucks		40	11.7	0		19.2	50	81.7		0		77.1						0	14.7		24	0		15.4	22.4
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	

# Ontario Traffic Inc.

### **Peak Hour Diagram**

**Specified Period** 

**One Hour Peak** 

From: To: 10:00:00 12:00:00

From: To: 11:00:00 12:00:00

**Intersection:** Bowmanville Ave & Energy Dr

 Site ID:
 1937500002

 Count Date:
 Nov 19, 2019

**Weather conditions:** 

\*\* Unsignalized Intersection \*\*

Major Road: Energy Dr runs E/W

#### **North Approach**

	Out	In	Total
	173	301	474
MT	44	71	115
HT	54	100	154
	271	472	743

#### **Bowmanville Ave**

,	4	1	Ú
Totals	227	44	0
	169	4	0
MT	41	3	0
HT	17	37	0

#### **Energy Dr**

	Totals		MT	HT	
7	0	0	0	0	
4	407	289	64	54	
7	19	4	2	13	

Out

293

66

67

426

MT

HT

**West Approach** 

In

170

42

23

235

Total

463

108

90

661

Peds: 0



Peds: 0

#### 

#### **Bowmanville Ave**

#### **South Approach**

	Out	In	Total
	13	8	21
MT	8	5	13
НТ	52	50	102
	73	63	136



MT - Medium Trucks

HT - Heavy Trucks

#### **Comments**



## **Peak Hour Summary**

Intersection: Bowmanville Ave & Energy Dr

Count Date: Nov 19, 2019

Period: 10:00 - 12:00

### Peak Hour Data (11:00 - 12:00)

		N Be	lorth A owman	pproac	ch ve			B	outh <i>F</i> owma	Approac nville A	h ve				East A	pproacl	1			,	West A Ener	pproac	h		Total Vehicl
Start Time	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	es
11:00		6	57	0	0	63	4	13		0	0	17					0		88		4	0	0	92	172
11:15		11	45	0	0	56	0	17		0	0	17					0		92		4	0	0	96	169
11:30		12	58	0	0	70	1	13		0	0	14					0		101		4	0	0	105	189
11:45		15	67	0	0	82	3	22		0	0	25					0		126		7	0	0	133	240
Grand Total		44	227	0	0	271	8	65		0	0	73					0	0	407		19	0	0	426	770
Approach %		16.2	83.8	0		-	11	89		0		-						-	95.5		4.5	0		-	
Totals %		5.7	29.5	0		35.2	1	8.4		0		9.5						0	52.9		2.5	0		55.3	
PHF		0.73	0.85	0		0.83	0.5	0.74		0		0.73						0	0.81		0.68	0		0.8	0.8
Cars		4	169	0		173	1	12		0		13						0	289		4	0		293	479
% Cars		9.1	74.4	0		63.8	12.5	18.5		0		17.8						0	71		21.1	0		68.8	62.2
Medium Trucks		3	41	0		44	1	7		0		8						0	64		2	0		66	118
% Medium Trucks		6.8	18.1	0		16.2	12.5	10.8		0		11						0	15.7		10.5	0		15.5	15.3
Heavy Trucks		37	17	0		54	6	46		0		52						0	54		13	0		67	173
% Heavy Trucks		84.1	7.5	0		19.9	75	70.8		0		71.2						0	13.3		68.4	0		15.7	22.5
Peds % Peds					0	-					0	-					0	-					0	-	0

#### Merkoulovitch, Ilia

**To:** Merkoulovitch, Ilia **Subject:** RE: Traffic Impact Study

From: Schmied, Sarah

Sent: September 18, 2019 8:18 AM

To: Sher, Ilya < ilya.sher@aecom.com >
Subject: RE: Traffic Impact Study

Good morning Ilya,

I posed your questions to the client and they provided the responses below in red. Please let me know if this information is sufficient for you to develop a scope of work and fee. Regarding question #2, please also advise if a count of trucks would be required.

I also looked into our contracts and it seems that AECOM/Golder have a MSA. I'm just confirming with our legal team if that would be considered a suitable contract for this scope of work and will try to get back to you as soon as possible. I assume that this would just be a desktop study, and that a site visit would not be required?

Feel free to let me know if you have any further questions.

Thank you,

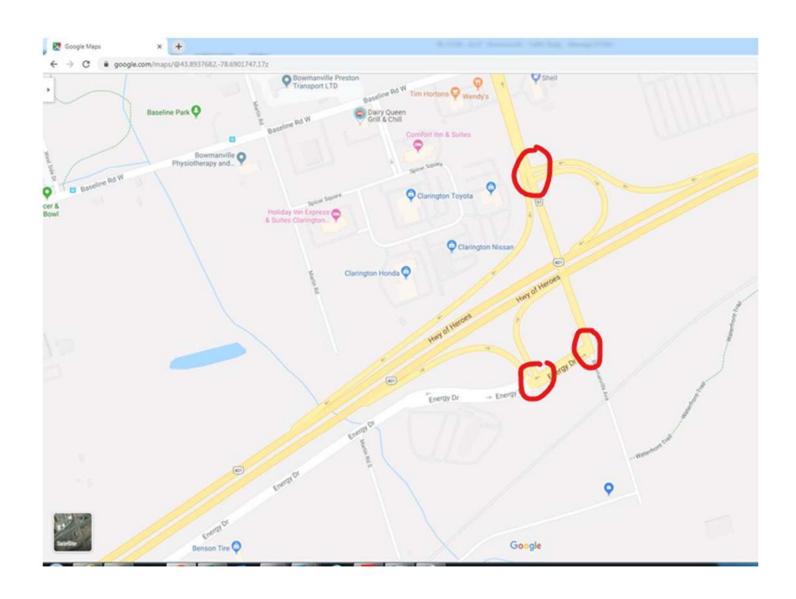
#### -Sarah

- 1. Operational characteristics of the site (hours of operation, busiest time-frames)

  The site operates 24/7. We truck from 4 am to 8 pm Monday to Friday but for modelling and flexibility purposes we assume it happens at any time. If there might be some potential problems/restrictions with the operation time please let me know.
- 2. Truck distribution if known (including types of trucks)
  The types of trucks we have coming into the site are dump trucks, life bottom trailers, stone slingers, tankers.
  We don't have a count of trucks. Do we need one? Inside the site we also have big loaders for quarry operation, but those stay onsite.
- 3. Will the increased use of ALCFs result in additional staff traffic?
- 4. Which intersections you would like assessed?

  The most relevant intersections would be the auxiliary road from the 401 to energy drive, energy drive and Bowmanville avenue, and Bowmanville avenue to the auxiliary road to the 401. See circles in red. These are the way the trucks will get in and outside the site. The trucks will drive inside the site, away from the site fence line.
- 5. Please confirm that the increase from 5 truck to 20 trucks is "per day" and not "per hour"

  Normally it will be 20 trucks per day (400 tonnes per day / 20 tonnes per truck = 20 trucks per day). There will be some days when we will need to receive more trucks to put material into storage, up to 35 trucks per day (400 tonnes per day \* 7 days consumption per week / 20 tonnes per truck \* 4 days per week delivery in a short week = 35 trucks per day). Having a conversation with production they will also want to have storage material for a long weekend, so that's 3 days' worth of material, 400 tonnes \* 3 = 1200 tonnes storage system.





#### Sarah Schmied (B.Sc, B.Ed) Environmental Assessment Specialist

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**A**ECOM

Appendix **B** 

**Synchro Reports** 

Intersection						
Int Delay, s/veh	14.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	₩.	WOR	4	HUIN	UDL	<u> </u>
Traffic Vol, veh/h	99	325	331	17	0	320
Future Vol, veh/h	99	325	331	17	0	320
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	310p	None	-	None	-	None
Storage Length	0	-	_	-	_	TVOTIC
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
	89		89			89
Peak Hour Factor		89		89	89	
Heavy Vehicles, %	3	3	8	71	0	9
Mvmt Flow	111	365	372	19	0	360
Major/Minor	Minor1	N	Major1	Λ	/lajor2	
Conflicting Flow All	742	382	0	0		-
Stage 1	382	-	-	-	_	_
Stage 2	360	_	_	_	_	_
Critical Hdwy	6.43	6.23	_	_	_	_
Critical Hdwy Stg 1	5.43	-	_	_	_	_
Critical Hdwy Stg 2	5.43	_		_	_	_
Follow-up Hdwy	3.527	3.327	-	-	-	-
Pot Cap-1 Maneuver	382	663		_	0	-
	688	- 003	-	-		-
Stage 1			-		0	
Stage 2	704	-	-	-	0	-
Platoon blocked, %	202	///	-	-		-
Mov Cap-1 Maneuver		663	-	-	-	-
Mov Cap-2 Maneuver	382	-	-	-	-	-
Stage 1	688	-	-	-	-	-
Stage 2	704	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s			0		0	
			U		U	
HCM LOS	Е					
Minor Lane/Major Mvr	nt	NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		_	_	566	-	
HCM Lane V/C Ratio		_	_	0.842	_	
HCM Control Delay (s	)	_		36.4	_	
HCM Lane LOS	,	-		30.4 E	-	
HCM 95th %tile Q(veh	1)	-	-	8.9	-	
HOW FOUT FOUTE Q(VEI	7	-	-	0.7	-	

Intersection						
Int Delay, s/veh	6.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ች	7	*	<b>↑</b>	ĵ.	
Traffic Vol, veh/h	318	26	8	30	66	353
Future Vol, veh/h	318	26	8	30	66	353
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	1050	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	6	23	38	67	20	5
Mvmt Flow	335	27	8	32	69	372
N A = 1 = 11/N A1 = 11	N 4' O		4-!1		1-10	
	Minor2		Major1		Major2	
Conflicting Flow All	303	255	441	0	-	0
Stage 1	255	-	-	-	-	-
Stage 2	48	-	-	-	-	-
Critical Hdwy	6.46	6.43	4.48	-	-	-
Critical Hdwy Stg 1	5.46	-	-	-	-	-
Critical Hdwy Stg 2	5.46	-	-	-	-	-
Follow-up Hdwy	3.554	3.507		-	-	-
Pot Cap-1 Maneuver	680	735	952	-	-	-
Stage 1	778	-	-	-	-	-
Stage 2	964	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	675	735	952	-	-	-
Mov Cap-2 Maneuver	675	-	-	-	-	-
Stage 1	772	-	-	-	-	-
Stage 2	964	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	15.1		1.9		0	
HCM LOS	C		1.7		U	
HOW LOS	C					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1 l	EBLn2	SBT
Capacity (veh/h)		952	-	675	735	-
HCM Lane V/C Ratio		0.009	-	0.496	0.037	-
HCM Control Delay (s)		8.8	-	15.5	10.1	-
HCM Lane LOS		Α	-	С	В	-
HCM 95th %tile Q(veh	)	0	-	2.8	0.1	-

Intersection							
Int Delay, s/veh	6.7						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		414	7>		<u> </u>	7	
Traffic Vol, veh/h	3	35	143	218	309	3	
Future Vol, veh/h	3	35	143	218	309	3	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	0	
Veh in Median Storage	,# -	0	0	-	0	-	
Grade, %	- 0F	0	0	- 0F	0	- 0F	
Peak Hour Factor	95 0	95 17	95 3	95 8	95 6	95 0	
Heavy Vehicles, % Mvmt Flow	3	37	151	229	325	3	
IVIVIIIL FIUW	3	31	101	229	323	J	
	/lajor1		Major2		Minor2		
Conflicting Flow All	380	0	-	0	291	266	
Stage 1	-	-	-	-	266	-	
Stage 2	-	-	-	-	25	- ( )	
Critical Hdwy Critical Hdwy Stg 1	4.1	-	-	-	6.69 5.49	6.2	
Critical Hdwy Stg 2	-	-	-	-	5.89	-	
Follow-up Hdwy	2.2	_	_		3.557	3.3	
Pot Cap-1 Maneuver	1190	_	_	_	678	778	
Stage 1	-	-	_	-	767	-	
Stage 2	-	-	-	-	984	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1190	-	-	-	676	778	
Mov Cap-2 Maneuver	-	-	-	-	676	-	
Stage 1	-	-	-	-	765	-	
Stage 2	-	-	-	-	984	-	
Approach	EB		WB		SB		
HCM Control Delay, s	0.6		0		15.1		
HCM LOS					С		
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR	SBLn1 :	SBI n2
Capacity (veh/h)		1190		-	-	676	778
HCM Lane V/C Ratio		0.003	-	_	_	0.481	
HCM Control Delay (s)		8	0	-	-	15.2	9.6
HCM Lane LOS		A	A	-	-	C	A
HCM 95th %tile Q(veh)		0	-	-	-	2.6	0
,							

Intersection Int Delay, s/veh						
	6					
		WED	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	070	700	10	0	200
Traffic Vol, veh/h	6	272	702	18	0	390
Future Vol, veh/h	6	272	702	18	0	390
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	0	1	2	6	0	3
Mvmt Flow	6	278	716	18	0	398
Major/Minor N	Minor1	N	/lajor1	N	/lajor2	
Conflicting Flow All	1123	725	0	0	-	
Stage 1	725	-	-	-	_	_
Stage 2	398	_	_	_	_	_
Critical Hdwy	6.4	6.21	_	_	_	_
Critical Hdwy Stg 1	5.4	-	_	_	_	_
Critical Hdwy Stg 2	5.4	-	_	_	_	_
Follow-up Hdwy	3.5	3.309	_	_	_	_
Pot Cap-1 Maneuver	230	427	_	_	0	_
Stage 1	483	-	_	_	0	_
Stage 2	683	_	_	_	0	_
Platoon blocked, %	003		_	_	U	_
	220				_	
May Can 1 Manauvar		127				
Mov Cap-1 Maneuver	230	427	-	-	-	-
Mov Cap-2 Maneuver	230	-	-	-	-	-
Mov Cap-2 Maneuver Stage 1	230 483	-	-	-	- -	-
Mov Cap-2 Maneuver	230	-	- - -	-	- - -	- - -
Mov Cap-2 Maneuver Stage 1	230 483	-	- - -	- - -	- - -	- - -
Mov Cap-2 Maneuver Stage 1	230 483	-	- - - -	-	- - - - SB	-
Mov Cap-2 Maneuver Stage 1 Stage 2  Approach	230 483 683	-	- - - - NB	-	- - -	-
Mov Cap-2 Maneuver Stage 1 Stage 2	230 483 683 WB	-		-	- - - SB	-
Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s	230 483 683 WB 29.8	-		-	- - - SB	-
Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS	230 483 683 WB 29.8 D	-	0	- - -	- - - SB 0	-
Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvml	230 483 683 WB 29.8 D	- - - NBT	0 NBRW	- - - - VBLn1	SB 0	
Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvml Capacity (veh/h)	230 483 683 WB 29.8 D	- - - NBT	0 NBRV	419	- - - SB 0	
Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	230 483 683 WB 29.8 D	NBT	0 NBRV -	419 0.677	- - - SB 0	
Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvml Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	230 483 683 WB 29.8 D	NBT -	0 NBRV	419 0.677 29.8	SB 0 SBT -	
Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	230 483 683 WB 29.8 D	NBT	0 NBRV -	419 0.677	- - - SB 0	

Intersection								
Int Delay, s/veh	37.9							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	ኘ	7	ሻ	<b>†</b>	\$	ODIT		
Traffic Vol, veh/h	665	7	26	55	4	392		
Future Vol, veh/h	665	7	26	55	4	392		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	- -	None		None	-	None		
Storage Length	0	0	1050	-	_	-		
Veh in Median Storag		-	1000	0	0	_		
Grade, %	0	_	_	0	0	_		
Peak Hour Factor	98	98	98	98	98	98		
Heavy Vehicles, %	1	57	4	13	0	3		
	679	7	27	56	4			
Mvmt Flow	0/9	1	21	00	4	400		
Major/Minor	Minor2		Major1	N	/lajor2			
Conflicting Flow All	314	204	404	0		0		
Stage 1	204	-	_	-	-	_		
Stage 2	110	-	-	_	_	-		
Critical Hdwy	6.41	6.77	4.14	_	_	_		
Critical Hdwy Stg 1	5.41	-	-	_	_	_		
Critical Hdwy Stg 2	5.41	_	_	_	_	_		
Follow-up Hdwy	3.509	3.813	2 236	_	_	_		
Pot Cap-1 Maneuver	681	715	1144	_	_	_		
Stage 1	833	713	1177	_		_		
Stage 2	917			<del>-</del>	_	_		
Platoon blocked, %	717	-	-	-	-	-		
Mov Cap-1 Maneuver	445	715	1144	-	-	-		
Mov Cap-1 Maneuver Mov Cap-2 Maneuver		710	1144	-	-			
		-	-	-	-	-		
Stage 1	813	-	-	-	-	-		
Stage 2	917	-	-	-	-	-		
Approach	EB		NB		SB			
HCM Control Delay, s	64.5		2.6		0			
HCM LOS	F							
= 5 5								
Minor Lane/Major Mvr	nt	NBL	NBT	EBLn1 E		SBT	SBR	
Capacity (veh/h)		1144	-	665	715	-	-	
HCM Lane V/C Ratio		0.023	-	1.02	0.01	-	-	
HCM Control Delay (s	5)	8.2	-	65.1	10.1	-	-	
HCM Lane LOS		Α	-	F	В	-	-	
HCM 95th %tile Q(veh	1)	0.1	-	16.8	0	-	-	
Votes		φ		1.6	20	0	LU NIBO	* All
-: Volume exceeds ca	apacity	\$: De	elay exc	eeds 30	JUS	+: Comp	outation Not Defined	*: All major volume in platoon

Intersection							
Int Delay, s/veh	15.9						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		414	1	W.D.K	<del></del> ንይደ	7	
Traffic Vol, veh/h	16	171	56	362	501	5	
Future Vol, veh/h	16	171	56	362	501	5	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	0	
Veh in Median Storage	e,# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	97	97	97	97	97	97	
Heavy Vehicles, %	0	1	4	3	2	0	
Mvmt Flow	16	176	58	373	516	5	
Major/Minor	Major1	N	Major2	N	Minor2		
Conflicting Flow All	431	0	-	0	365	245	
Stage 1	431	-		-	245	243	
Stage 2	_	-	_	_	120	_	
Critical Hdwy	4.1	-	-	-	6.63	6.2	
Critical Hdwy Stg 1	-	-	-	-	5.43	-	
Critical Hdwy Stg 2	-	-	-	-	5.83	-	
Follow-up Hdwy	2.2	-	-	-	3.519	3.3	
Pot Cap-1 Maneuver	1139	-	-	-	621	799	
Stage 1	-	-	-	-	795	-	
Stage 2	-	-	-	-	893	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1139	-	-	-	611	799	
Mov Cap-2 Maneuver	-	-	-	-	611	-	
Stage 1	-	-	-	-	782	-	
Stage 2	-	-	-	-	893	-	
Approach	EB		WB		SB		
	0.8		0		34.6		
HCM Control Delay, s HCM LOS	0.0		U				
HOW LUS					D		
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1 S	
Capacity (veh/h)		1139	-	-	-	611	799
HCM Lane V/C Ratio		0.014	-	-	-	0.845	
HCM Control Delay (s	)	8.2	0.1	-	-	0	9.5
HCM Lane LOS		Α	Α	-	-	D	Α
HCM 95th %tile Q(veh	1)	0	-	-	-	9.2	0

Intersection						
Int Delay, s/veh	20.9					
		14/55	NET	NES	05:	05=
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		₽			
Traffic Vol, veh/h	106	348	354	19	0	342
Future Vol, veh/h	106	348	354	19	0	342
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	89	89	89	89	89	89
Heavy Vehicles, %	3	3	8	68	0	9
Mymt Flow	119	391	398	21	0	384
IVIVIII( I IOVV	117	371	370	۷ ۱	U	304
Major/Minor	Minor1	Ν	/lajor1	Λ	/lajor2	
Conflicting Flow All	793	409	0	0	-	-
Stage 1	409	-	-	-	-	-
Stage 2	384	-	_	-	-	-
Critical Hdwy	6.43	6.23	_	_	-	_
Critical Hdwy Stg 1	5.43	-	_	_	_	_
Critical Hdwy Stg 2	5.43	_	_	_	_	_
Follow-up Hdwy	3.527	3.327		_	_	_
Pot Cap-1 Maneuver	356	640	-	-	0	-
•	668	- 040	-	-	0	_
Stage 1			-			
Stage 2	686	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		640	-	-	-	-
Mov Cap-2 Maneuver	356	-	-	-	-	-
Stage 1	668	-	-	-	-	-
Stage 2	686	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	53.9		0		0	
HCM LOS	55.7 F		U		U	
HCIVI LUS	Г					
Minor Lane/Major Mvr	nt	NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		_	_	540	_	
HCM Lane V/C Ratio		_	_	0.945	_	
HCM Control Delay (s	)	_		53.9	-	
HCM Lane LOS		_		55.7 F	-	
	,)		-			
HCM 95th %tile Q(veh	I)	-	-	12.1	-	

Intersection							
Int Delay, s/veh	7.1						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	LDL	LDK.	NDL		3B1 <b>}</b>	אטכ	
Traffic Vol, veh/h	340	27	8	<b>↑</b> 33	70	378	
Future Vol, veh/h	340	27	8	33	70	378	
	340	0	0	0	0	0	
Conflicting Peds, #/hr			Free	Free	Free	Free	
Sign Control RT Channelized	Stop -	Stop None		None	riee -	None	
	0	0	1050	None -	-	None	
Storage Length		-	1000		0	-	
Veh in Median Storage				0	0	-	
Grade, %	0	- 0F	- 0F	0	0	- 0F	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	6	22	38	67	20	5	
Mvmt Flow	358	28	8	35	74	398	
Major/Minor	Minor2	1	Major1	ľ	Major2		
Conflicting Flow All	324	273	472	0	-	0	
Stage 1	273	-	-	-	_	-	
Stage 2	51	-	-	-	-	_	
Critical Hdwy	6.46	6.42	4.48	-	_	-	
Critical Hdwy Stg 1	5.46	-	-	-	-	_	
Critical Hdwy Stg 2	5.46	_	_	_	-	-	
Follow-up Hdwy	3.554	3.498	2.542	_	-	-	
Pot Cap-1 Maneuver	662	720	926	_	_	-	
Stage 1	764	-	-	-	-	-	
Stage 2	961	_	_	_	-	_	
Platoon blocked, %	701			_	_	_	
Mov Cap-1 Maneuver	656	720	926	_	_	_	
Mov Cap 1 Maneuver	656	720	720	_	_	_	
Stage 1	757	_	_	_	_	_	
Stage 2	961	_	_	_	_	_	
Stage 2	701						
Approach	EB		NB		SB		
HCM Control Delay, s	16.4		1.7		0		
HCM LOS	С						
Minor Lane/Major Mvn	nt	NBL	MRTI	EBLn1 I	FRI n2	SBT	
WILLOU Lanchwajor WWI	iii.	926	-		720	301	
						-	
Capacity (veh/h)				() 6/16		_	
Capacity (veh/h) HCM Lane V/C Ratio	١	0.009		0.546			
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	)	0.009 8.9	-	16.9	10.2	-	
Capacity (veh/h) HCM Lane V/C Ratio		0.009		16.9 C			

7.2						
FRI	FRT	WRT	WRR	SRI	SRR	
LDL			אטוע			
2			234			
				•		
		_				
		0				
J	37	100	240	J4 <i>1</i>	J	
Major1	N	Major2	N	Minor2		
406	0	-	0	309	283	
-	-	-	-	283	-	
-	-	-	-	26	-	
4.1	-	-	-	6.69	6.2	
-	-	-	-	5.49	-	
-	-	-	-	5.89	-	
2.2	-	-	-	3.557	3.3	
1164	-	-	-	661	761	
-	-	-	-	754	-	
-	-	-	-	982	-	
	-	-	-			
1164	-	-	-	659	761	
-	-	-	-	659	-	
-	-	-	-	752	-	
-	-	-	-	982	-	
ED		MD		CD		
0.6		0		_		
				C		
nt	EBL	EBT	WBT	WBR :	SBLn1	SBLn2
						761
						9.8
						7.0 A
١						0
,	U				J. I	U
	EBL  3 3 0 Free 95 0 3  Major1 406 4.1 - 2.2 1164 1164 EB 0.6	BBL EBT  3 37 3 37 0 0 0 Free Free - None 0 95 95 0 16 3 39  Major1 N  406 0 4.1 2.2 - 1164	EBL EBT WBT  3 37 152 3 37 152 0 0 0 0 Free Free Free - None 2 # - 0 0 95 95 95 0 16 3 3 39 160  Major1 Major2  406 0 4.1 2.2 1164 2.2 1164  1164  1164  EB WB 0.6 0  at EBL EBT  1164 - 0	EBL EBT WBT WBR  3 37 152 234 3 37 152 234 0 0 0 0 0 Free Free Free Free - None	EBL         EBT         WBT         WBR         SBL           3         37         152         234         330           0         0         0         0         0           Free         Free         Free         Stop           None         -         None         -           -         0         0         -         0           -         0         0         -         0           95         95         95         95         95           0         16         3         8         6           3         39         160         246         347           Major1         Major2         Minor2           406         0         -         0         309           -         -         -         283         -         -         283           -         -         -         -         26         4.1         -         -         6.69           -         -         -         -         5.49         -         -         -         5.89           2.2         -         -         -         -         659 <t< td=""><td>  EBL   EBT   WBT   WBR   SBL   SBR    </td></t<>	EBL   EBT   WBT   WBR   SBL   SBR

Intersection						
Int Delay, s/veh	7.9					
		WED	NET	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		ĵ.			<b>^</b>
Traffic Vol, veh/h	6	291	751	19	0	418
Future Vol, veh/h	6	291	751	19	0	418
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	0	1	2	5	0	3
Mvmt Flow	6	297	766	19	0	427
		_,,	, , ,			,
	Minor1		/lajor1	Λ	/lajor2	
Conflicting Flow All	1203	776	0	0	-	-
Stage 1	776	-	-	-	-	-
Stage 2	427	-	-	-	-	-
Critical Hdwy	6.4	6.21	-	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	_	_	-	_	-
Follow-up Hdwy	3.5	3.309	_	_	_	_
Pot Cap-1 Maneuver	206	399	_	-	0	_
Stage 1	457	-	_	_	0	_
Stage 2	662	_		_	0	_
Platoon blocked, %	002	-	-		U	
	207	200	-	-		-
Mov Cap-1 Maneuver	206	399	-	-	-	-
Mov Cap-2 Maneuver	206	-	-	-	-	-
Stage 1	457	-	-	-	-	-
Stage 2	662	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	39.3		0		0	
HCM LOS	57.5 E		U		U	
TICIVI LOS						
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	392	_	
HCM Lane V/C Ratio		_	_	0.773	-	
HCM Control Delay (s)	)	-	-	39.3	-	
HCM Lane LOS		_	_	57.5 E	_	
HCM 95th %tile Q(veh	1)			6.5		
HOW FOUT WILLS (VEI)	I)	_	_	0.0	-	

Intersection								
nt Delay, s/veh	57.7							
Novement	EBL	EBR	NBL	NBT	SBT	SBR		
ane Configurations	ች	7	ች	<b></b>	₽			
Fraffic Vol, veh/h	711	7	28	59	4	420		
uture Vol, veh/h	711	7	28	59	4	420		
onflicting Peds, #/hr		0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized		None	-	None	-	None		
Storage Length	0	0	1050	-	-	-		
/eh in Median Storag		-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	98	98	98	98	98	98		
Heavy Vehicles, %	1	57	4	12	0	3		
/lvmt Flow	726	7	29	60	4	429		
Major/Minor	Minor2	1	Major1	N	/lajor2			
Conflicting Flow All	337	219	433	0	- najoiz	0		
Stage 1	219	217	433	-		-		
Stage 2	118	_	_	_	_	_		
Critical Hdwy	6.41	6.77	4.14	_	_	_		
ritical Hdwy Stg 1	5.41	0.77	- 1.17	_	_	_		
Critical Hdwy Stg 2	5.41	_	_	_	_	_		
follow-up Hdwy	3.509	3.813	2.236	_	_	_		
Pot Cap-1 Maneuver	~ 661	701	1116	_	_	_		
Stage 1	820	-				_		
Stage 2	910	-	-	-	-	-		
Platoon blocked, %	,13			_		_		
Nov Cap-1 Maneuver	~ 644	701	1116	-	-	-		
Nov Cap 1 Maneuver			-	-	-	-		
Stage 1	799	-	-	-	-	-		
Stage 2	910	_	_	_	_	_		
g	,.5							
.pproach	EB		NB		SB			
HCM Control Delay, s			2.7		0			
HCM CONTROL Delay, 3 HCM LOS	F		۷.۱		U			
TOWN EOO								
Minor Lane/Major Mvi	mt	NBL	NBT	EBLn1 E	BLn2	SBT	SBR	
Capacity (veh/h)		1116		644	701		-	
ICM Lane V/C Ratio		0.026	_	1.127	0.01	_	-	
HCM Control Delay (s	;)	8.3	_	99.4	10.2	_	-	
ICM Lane LOS	,	Α	_	77.4 F	В	_	-	
ICM 95th %tile Q(vel	n)	0.1	-	22.4	0	-	-	
·	,	0.1			J			
lotes		<b>#</b> 5			20		LU NIBO	* AH
: Volume exceeds ca	apacity	\$: De	elay exc	eeds 30	J0s	+: Com	putation Not Defined	*: All major volume in platoon

Intersection							
Int Delay, s/veh	23						
		EDT	WOT	MDD	CDI	CDD	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	47	4↑	<b>^</b>	000	<b>\</b>	7	
Traffic Vol, veh/h	17	182	60	388	536	5	
Future Vol, veh/h	17	182	60	388	536	5	
Conflicting Peds, #/hr	0	0	0	0	0	O Cton	
Sign Control RT Channelized	Free -	Free None	Free	Free	Stop	Stop	
		None -	-		-	None	
Storage Length	-		-	-	0	0	
Veh in Median Storage		0	0	-	0	-	
Grade, %	- 07	0	0	- 07	0	- 07	
Peak Hour Factor	97	97	97	97	97	97	
Heavy Vehicles, %	0 18	100	3 62	3	553	0	
Mvmt Flow	Ιğ	188	02	400	223	5	
Major/Minor I	Major1	N	/lajor2		Minor2		
Conflicting Flow All	462	0	-	0	392	262	
Stage 1	-	-	-	-	262	-	
Stage 2	-	-	-	-	130	-	
Critical Hdwy	4.1	-	-	-	6.63	6.2	
Critical Hdwy Stg 1	-	-	-	-	5.43	-	
Critical Hdwy Stg 2	-	-	-	-	5.83	-	
Follow-up Hdwy	2.2	-	-	-	3.519	3.3	
Pot Cap-1 Maneuver	1110	-	-	-	598	782	
Stage 1	-	-	-	-	781	-	
Stage 2	-	-	-	-	883	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1110	-	-	-	587	782	
Mov Cap-2 Maneuver	-	-	-	-	587	-	
Stage 1	-	-	-	-	767	-	
Stage 2	-	-	-	-	883	-	
Annroach	EB		WB		SB		
Approach							
HCM Control Delay, s	8.0		0		50.2		
HCM LOS					F		
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	SBLn1 S	BLn2
Capacity (veh/h)		1110	-		-	587	782
HCM Lane V/C Ratio		0.016	-	-	-	0.941	
HCM Control Delay (s)		8.3	0.1	-	-	50.6	9.6
HCM Lane LOS		Α	Α	-	-	F	Α
HCM 95th %tile Q(veh)	)	0	-	-	-	12.4	0
	,	-					_

Int Delay, s/veh	Intersection						
Movement		24					
Lane Configurations			WED	NET	NDD	CDI	CDT
Traffic Vol, veh/h			WBK		NRK	SRF	
Future Vol, veh/h			0.10		46	-	
Conflicting Peds, #/hr   O   O   O   O   O   O   Sign Control   Stop   Stop   Free   Free							
Sign Control         Stop RT Channelized         Stop RT Channelized         Stop RT Channelized         None         No         No <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>							
RT Channelized         - None         None         None         None           Storage Length         0         - 0         - 0         - 0           Veh in Median Storage, # 0         - 0         - 0         - 0         - 0           Grade, %         0         - 0         - 0         - 0           Peak Hour Factor         89         89         89         89         89           Heavy Vehicles, %         9         3         8         68         0         9           Mwmt Flow         127         391         398         21         0         384           Morright         Major1         Major2         Major3         409         - 0         - 0							
Storage Length		Stop		Free		Free	
Veh in Median Storage, #         0         -         0         -         -         0           Grade, %         0         -         0         -         -         0           Peak Hour Factor         89         89         89         89         89         89           Heavy Vehicles, %         9         3         8         68         0         9           Mvmt Flow         127         391         398         21         0         384           Major/Minor         Minor         Major/Minor			None	-	None	-	None
Grade, %         0         -         0         -         -         0           Peak Hour Factor         89         80         80         90         0			-	-	-	-	-
Peak Hour Factor         89         80           Mow right         Minor Lane/Major Mull         127         391         398         21         0         384           Conflicting Flow All         793         409         0 <td></td> <td>e, # 0</td> <td>-</td> <td>0</td> <td>-</td> <td>-</td> <td>0</td>		e, # 0	-	0	-	-	0
Heavy Vehicles, %   9   3   8   68   0   9   Mvmt Flow   127   391   398   21   0   384	Grade, %	0	-	0	-	-	0
Mymt Flow         127         391         398         21         0         384           Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         793         409         0         0         -         -           Stage 1         409         -	Peak Hour Factor	89	89	89	89	89	89
Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         793         409         0         0         -         -           Stage 1         409         -	Heavy Vehicles, %	9	3	8	68	0	9
Conflicting Flow All       793       409       0       0       -       -         Stage 1       409       -       -       -       -       -         Stage 2       384       -       -       -       -       -         Critical Hdwy       6.49       6.23       -       -       -       -         Critical Hdwy Stg 1       5.49       -		127	391	398	21	0	384
Conflicting Flow All       793       409       0       0       -       -         Stage 1       409       -       -       -       -       -         Stage 2       384       -       -       -       -       -         Critical Hdwy       6.49       6.23       -       -       -       -         Critical Hdwy Stg 1       5.49       -							
Conflicting Flow All         793         409         0         0         -         -           Stage 1         409         -         <	Naion/Naion	N 11: 1		1-11		1-:2	
Stage 1       409       -       -       -       -         Stage 2       384       -       -       -       -         Critical Hdwy       6.49       6.23       -       -       -         Critical Hdwy Stg 1       5.49       -       -       -       -         Critical Hdwy Stg 2       5.49       -       -       -       -         Follow-up Hdwy       3.581       3.327       -       -       -       -         Follow-up Hdwy       3.581       3.327       -        -       <							
Stage 2       384       -			409	0	0	-	-
Critical Hdwy       6.49       6.23       -       -       -         Critical Hdwy Stg 1       5.49       -       -       -       -         Critical Hdwy Stg 2       5.49       -       -       -       -         Follow-up Hdwy       3.581       3.327       -       -       -       -         Pot Cap-1 Maneuver       348       640       -       0       -         Stage 1       656       -       -       0       -         Stage 2       673       -       -       0       -         Mov Cap-1 Maneuver       348       640       -       -       -         Mov Cap-2 Maneuver       348       -       -       -       -         Stage 1       656       -       -       -       -         Stage 2       673       -       -       -       -         Approach       WB       NB       NB         HCM Control Delay, s       61.1       0       0         Minor Lane/Major Mvmt       NBT       NBRWBLn1       SBT         Capacity (veh/h)       -       -       531       -         HCM Control Delay (s)       -			-	-	-	-	-
Critical Hdwy Stg 1 5.49				-	-	-	-
Critical Hdwy Stg 2 5.49 Follow-up Hdwy 3.581 3.327			6.23	-	-	-	-
Follow-up Hdwy 3.581 3.327	Critical Hdwy Stg 1		-	-	-	-	-
Pot Cap-1 Maneuver         348         640         -         0         -           Stage 1         656         -         -         0         -           Stage 2         673         -         -         0         -           Platoon blocked, %         -         -         -         -         -           Mov Cap-1 Maneuver         348         640         -         -         -         -           Mov Cap-2 Maneuver         348         -         -         -         -         -         -           Stage 1         656         -<	Critical Hdwy Stg 2			-	-	-	-
Stage 1       656       -       -       0       -         Stage 2       673       -       -       0       -         Platoon blocked, %       -       -       -       -       -         Mov Cap-1 Maneuver       348       640       -       -       -       -         Mov Cap-2 Maneuver       348       -       -       -       -       -       -         Stage 1       656       - </td <td>Follow-up Hdwy</td> <td>3.581</td> <td>3.327</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td>	Follow-up Hdwy	3.581	3.327	-	-	-	-
Stage 1       656       -       -       0       -         Stage 2       673       -       -       0       -         Platoon blocked, %       -       -       -       -       -         Mov Cap-1 Maneuver       348       640       -       -       -       -         Mov Cap-2 Maneuver       348       -       -       -       -       -       -         Stage 1       656       -       -       -       -       -       -       -         Stage 2       673       -       -       -       -       -       -       -         Approach       WB       NB       NB       SB         HCM Control Delay, s       61.1       0       0       0         HCM LOS       F     Minor Lane/Major Mvmt  NBT NBRWBLn1  SBT  Capacity (veh/h)  - 531 - HCM Lane V/C Ratio - 0.975 - HCM Control Delay (s) - 61.1 - 0.975 - HCM Control Delay (s) - 61.1 - 0.975 - 0	Pot Cap-1 Maneuver	348	640	-	-	0	-
Stage 2       673       -       -       0       -         Platoon blocked, %       -       -       -       -       -         Mov Cap-1 Maneuver       348       640       -       -       -       -         Mov Cap-2 Maneuver       348       -       -       -       -       -       -         Stage 1       656       -		656	-	-	-	0	-
Platoon blocked, %			-	-	-	0	-
Mov Cap-1 Maneuver         348         640         -				_	-		-
Mov Cap-2 Maneuver         348         -		348	640	-	-	-	-
Stage 1         656         -					_		_
Stage 2         673         -				_		_	_
Approach         WB         NB         SB           HCM Control Delay, s         61.1         0         0           HCM LOS         F           Minor Lane/Major Mvmt         NBT         NBRWBLn1         SBT           Capacity (veh/h)         -         -         531         -           HCM Lane V/C Ratio         -         -         0.975         -           HCM Control Delay (s)         -         61.1         -				_	_	_	_
HCM Control Delay, s 61.1 0 0  HCM LOS F  Minor Lane/Major Mvmt NBT NBRWBLn1 SBT  Capacity (veh/h) - 531 -  HCM Lane V/C Ratio - 0.975 -  HCM Control Delay (s) - 61.1 -	Stage 2	073					
HCM Control Delay, s 61.1 0 0  HCM LOS F  Minor Lane/Major Mvmt NBT NBRWBLn1 SBT  Capacity (veh/h) - 531 -  HCM Lane V/C Ratio - 0.975 -  HCM Control Delay (s) - 61.1 -							
HCM LOS F  Minor Lane/Major Mvmt NBT NBRWBLn1 SBT  Capacity (veh/h) 531 -  HCM Lane V/C Ratio - 0.975 -  HCM Control Delay (s) - 61.1 -	Approach	WB		NB		SB	
Minor Lane/Major Mvmt NBT NBRWBLn1 SBT  Capacity (veh/h) 531 -  HCM Lane V/C Ratio - 0.975 -  HCM Control Delay (s) - 61.1 -	HCM Control Delay, s	61.1		0		0	
Capacity (veh/h) 531 - HCM Lane V/C Ratio - 0.975 - HCM Control Delay (s) - 61.1 -	HCM LOS	F					
Capacity (veh/h) 531 - HCM Lane V/C Ratio - 0.975 - HCM Control Delay (s) - 61.1 -							
Capacity (veh/h) 531 - HCM Lane V/C Ratio - 0.975 - HCM Control Delay (s) - 61.1 -	N. A	-1	NDT	MDDW	VDI 1	CDT	
HCM Lane V/C Ratio - 0.975 - HCM Control Delay (s) - 61.1 -		II					
HCM Control Delay (s) 61.1 -			-				
			-			-	
HCM Lane LOS F -			-	-		-	
	HCM Lane LOS		-	-		-	
HCM 95th %tile Q(veh) 13.1 -					101		

Intersection						
Int Delay, s/veh	7.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	T T	LDK.	NDL	†	1 <sub>10</sub> C	אוטכ
Traffic Vol, veh/h	340	27	12	33	77	378
Future Vol, veh/h	340	27	12	33	77	378
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	310p	None	-		-	None
Storage Length	0	0	1050	None -	-	None
		-	1030		0	-
Veh in Median Storage				0	0	-
Grade, %	0	-	- 0F	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	6	22	58	67	27	5
Mvmt Flow	358	28	13	35	81	398
Major/Minor	Minor2		Major1	Λ	/lajor2	
Conflicting Flow All	341	280	479	0		0
Stage 1	280	-	-	-	-	-
Stage 2	61	_	_	_	_	_
Critical Hdwy	6.46	6.42	4.68	_	-	_
Critical Hdwy Stg 1	5.46	-	-	_	_	_
Critical Hdwy Stg 2	5.46	_	_	-	_	_
Follow-up Hdwy	3.554	3.498	2.722	_	_	_
Pot Cap-1 Maneuver	647	713	846	_	-	_
Stage 1	758	-	-	_	_	_
Stage 2	952	_	_	_	_	_
Platoon blocked, %	702			_	_	_
Mov Cap-1 Maneuver	637	713	846	_	_	_
Mov Cap-1 Maneuver	637	713	040	_	_	_
Stage 1	747				_	
ū	952	-	-	-	-	-
Stage 2	932	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	17.2		2.5		0	
HCM LOS	С					
HOW LOS						
HOW EOS						
		NIDI	NDT	CDI 51 F	בשום	CDT
Minor Lane/Major Mvm	nt	NBL		EBLn1 E		SBT
Minor Lane/Major Mvm Capacity (veh/h)	nt	846	-	637	713	-
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio		846 0.015	-	637 0.562	713 0.04	- -
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		846 0.015 9.3	-	637 0.562 17.7	713 0.04 10.3	- - -
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio		846 0.015	-	637 0.562 17.7 C	713 0.04	- -

Intersection							
Int Delay, s/veh	7.2						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	LDL		₩ <b>₽</b>	אטא	JDL	7 JUK	
Traffic Vol, veh/h	3	<b>₹</b>	152	238	330	3	
Future Vol, veh/h	3	37	152	238	330	3	
Conflicting Peds, #/hr	0	0	0	230	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	310p	None	
		NONE -	-	None -			
Storage Length Veh in Median Storage	- #	0	-		0	0	
			0	-	0	-	
Grade, %	- 0F	0	0	- 0F	0	- 0F	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	0	16	3	9	6	0	
Mvmt Flow	3	39	160	251	347	3	
Major/Minor	Major1		Major2	N	Minor2		
Conflicting Flow All	411	0	-	0	312	286	
Stage 1	-	-	_	-	286	-	
Stage 2	_	_	_	_	26	_	
Critical Hdwy	4.1	-	_	_	6.69	6.2	
Critical Hdwy Stg 1	-	_	_	_	5.49	-	
Critical Hdwy Stg 2	_	_	_		5.89	_	
Follow-up Hdwy	2.2	_	_	_	3.557	3.3	
Pot Cap-1 Maneuver	1159			_	659	758	
Stage 1	1107	-	-		751	750	
Stage 2	-	-	-		982	-	
Platoon blocked, %	-	-	-		702	-	
	1150	-	-	-	657	758	
Mov Cap-1 Maneuver	1159	-	-	-			
Mov Cap-2 Maneuver	-	-	-	-	657	-	
Stage 1	-	-	-	-	749	-	
Stage 2	-	-	-	-	982	-	
Approach	EB		WB		SB		
HCM Control Delay, s	0.6		0		16.4		
HCM LOS	0.0				C		
TIOW EOO					J		
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR:	SBLn1 S	
Capacity (veh/h)		1159	-	-	-	657	758
HCM Lane V/C Ratio		0.003	-	-	-	0.529	0.004
HCM Control Delay (s)		8.1	0	-	-	16.5	9.8
HCM Lane LOS		Α	Α	-	-	С	Α
HCM 95th %tile Q(veh	)	0	-	-	-	3.1	0

Intersection						
Int Delay, s/veh	8.3					
		MADD	NID	T NDD	CDI	CDT
Movement	WBL	WBR			SBL	SBT
Lane Configurations	¥	201		1 10	0	410
Traffic Vol, veh/h	8	291	75			418
Future Vol, veh/h	8	291	75			418
Conflicting Peds, #/hr	0	0		0 0		_ 0
Sign Control	Stop	Stop				Free
RT Channelized	-	None		- None		None
Storage Length	0	-			-	-
Veh in Median Storage		-		0 -	-	0
Grade, %	0	-		0 -		0
Peak Hour Factor	98	98				98
Heavy Vehicles, %	24	1		2 5		3
Mvmt Flow	8	297	76	6 19	0	427
Major/Minor	Minor1	N	Major	1	Major2	
Conflicting Flow All	1203	776		0 0		
Stage 1	776	-			_	_
Stage 2	427	_			_	_
Critical Hdwy	6.64	6.21			-	_
Critical Hdwy Stg 1	5.64	0.21			-	_
	5.64					-
Critical Hdwy Stg 2		3.309			-	-
Follow-up Hdwy	3.716				-	-
Pot Cap-1 Maneuver	184	399			0	-
Stage 1	417	-			0	-
Stage 2	614	-			0	-
Platoon blocked, %						-
Platoon blocked, % Mov Cap-1 Maneuver		399		 	-	-
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver	184	399 -		 	-	- - -
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1	184 417			  	- - -	- - -
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver	184	-		  	- - -	- - -
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1	184 417	-			- - -	- - -
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2	184 417 614	-		  	- - -	-
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2  Approach	184 417 614 WB	-	N		- - SB	
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s	184 417 614 WB 41.3	-	N		- - -	-
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2  Approach	184 417 614 WB	-	N		- - SB	-
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS	184 417 614 WB 41.3 E	-	N		- - SB	-
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s	184 417 614 WB 41.3 E	-	N		- - SB	
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS	184 417 614 WB 41.3 E	-	N NB	0	SB 0	
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvn	184 417 614 WB 41.3 E	- - - NBT	N NB	0 RWBLn1	SB 0	
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvn Capacity (veh/h)	184 417 614 WB 41.3 E	- - - NBT	N NB	0 RWBLn1 - 387	- - - SB 0	
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvn Capacity (veh/h) HCM Lane V/C Ratio	184 417 614 WB 41.3 E	- - - NBT	N NB	0 RWBLn1 - 387 - 0.788	- - - SB 0	
Platoon blocked, % Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvn Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	184 417 614 WB 41.3 E	NBT -	NB	0 RWBLn1 - 387 - 0.788 - 41.3		

Intersection								
Int Delay, s/veh	64.6							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	*	7	ች	<b>↑</b>	¢			
Traffic Vol, veh/h	711	7	34	59	6	420		
Future Vol, veh/h	711	7	34	59	6	420		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	0	1050	-	-	-		
Veh in Median Storage	, # 0	-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	98	98	98	98	98	98		
Heavy Vehicles, %	1	57	21	12	32	3		
Mvmt Flow	726	7	35	60	6	429		
Major/Minor N	Minor2		Major1		Major2			
Conflicting Flow All	351	221	435	0	viajuiz	0		
Stage 1	221	-		-	_	-		
Stage 2	130	_	_	_	_	_		
Critical Hdwy	6.41	6.77	4.31	-	_	_		
Critical Hdwy Stg 1	5.41	0.77	4.51	_		_		
Critical Hdwy Stg 2	5.41	_			_	_		
Follow-up Hdwy			2 380	_	_	_		
Pot Cap-1 Maneuver	~ 648	699	1031		_	_		
Stage 1	818	077	1031	_		_		
Stage 2	898	_	_		_	_		
Platoon blocked, %	070			_		_		
Mov Cap-1 Maneuver	- 626	699	1031		_	_		
Mov Cap-1 Maneuver		077	1031	_		_		
Stage 1	790	-	-	_	-	_		
Stage 2	898	-						
Jidge Z	070					-		
A	ED		NID.		CD			
Approach	110.0		NB		SB			
HCM Control Delay, s			3.1		0			
HCM LOS	F							
Minor Lane/Major Mvm	t	NBL	NBT	EBLn1 [		SBT	SBR	
Capacity (veh/h)		1031	-	626	699	-	-	
HCM Lane V/C Ratio		0.034		1.159	0.01	-	-	
HCM Control Delay (s)		8.6	-	111.9	10.2	-	-	
HCM Lane LOS		Α	-	F	В	-	-	
HCM 95th %tile Q(veh)		0.1	-	23.8	0	-	-	
Notes								
~: Volume exceeds cap	pacity	\$: De	elay exc	ceeds 30	00s	+: Comi	outation Not Defined	*: All major volume in platoon
	y	, 5				. 50.11		

Int Delay, s/veh 23.2  Movement EBL EBT WBT WBR SBL SBI	
	2
	•
	5
· ·	5
	0
Sign Control Free Free Free Stop Sto	
RT Channelized - None - None - None	
	0
	-
Grade, % - 0 0 - 0	_
Peak Hour Factor 97 97 97 97 97 9	
	0
· · · · · · · · · · · · · · · · · · ·	5
10 100 02 400 555	J
Major/Minor Major1 Major2 Minor2	
Conflicting Flow All 468 0 - 0 395 26	5
Stage 1 265	-
Stage 2 130	-
Critical Hdwy 4.1 6.63 6.	2
Critical Hdwy Stg 1 5.43	-
Critical Hdwy Stg 2 5.83	-
Follow-up Hdwy 2.2 3.519 3.	3
Pot Cap-1 Maneuver 1104 596 77	9
Stage 1 779	-
Stage 2 883	-
Platoon blocked, %	
Mov Cap-1 Maneuver 1104 585 77	9
Mov Cap-2 Maneuver 585	-
Stage 1 765	-
Stage 2 883	-
Approach ED WD CD	
Approach EB WB SB	
HCM Control Delay, s 0.8 0 50.9	
HCM LOS F	
Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn	1 SBLn2
Capacity (veh/h) 1104 58	
	5 0.007
HCM Control Delay (s) 8.3 0.1 51.	
	F A
HCM 95th %tile Q(veh) 0 12.	
12.	

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Lane Group	EBL	EBR	NBL	NBT	SBT
Lane Group Flow (vph)	358	28	8	35	472
v/c Ratio	0.53	0.05	0.03	0.07	0.50
Control Delay	17.6	5.4	9.6	9.6	4.3
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	17.6	5.4	9.6	9.6	4.3
Queue Length 50th (m)	29.1	0.0	0.5	2.0	4.4
Queue Length 95th (m)	50.6	3.9	2.4	6.1	18.7
Internal Link Dist (m)	80.4			124.5	290.2
Turn Bay Length (m)			105.0		
Base Capacity (vph)	674	541	233	527	942
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.53	0.05	0.03	0.07	0.50
Intersection Summary					

	ʹ		_	•	1	
		*	7	ı	*	*
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	7	7	7	<b>↑</b>	<b>₽</b>	
Traffic Volume (veh/h)	340	27	8	33	70	378
Future Volume (veh/h)	340	27	8	33	70	378
Number	7	14	5	2	6	16
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1792	1557	1377	1138	1770	1900
Adj Flow Rate, veh/h	358	28	8	35	74	398
Adj No. of Lanes	1	1	1	1	1	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	6	22	38	67	20	20
Cap, veh/h	669	518	269	521	111	595
Arrive On Green	0.39	0.39	0.46	0.46	0.46	0.46
Sat Flow, veh/h	1707	1324	678	1138	242	1299
Grp Volume(v), veh/h	358	28	8	35	0	472
Grp Sat Flow(s), veh/h/ln	1707	1324	678	1138	0	1541
Q Serve(g_s), s	9.7	0.8	0.6	1.0	0.0	14.4
Cycle Q Clear(g_c), s	9.7	0.8	14.9	1.0	0.0	14.4
Prop In Lane	1.00	1.00	1.00	1.0	0.0	0.84
Lane Grp Cap(c), veh/h	669	518	269	521	0	706
V/C Ratio(X)	0.54	0.05	0.03	0.07	0.00	0.67
Avail Cap(c_a), veh/h	669	518	269	521	0.00	706
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
		1.00	1.00	1.00		1.00
Upstream Filter(I)	1.00		18.5		0.00	
Uniform Delay (d), s/veh	14.0	11.3		9.1		12.7
Incr Delay (d2), s/veh	3.1	0.2	0.2	0.2	0.0	5.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.0	0.3	0.1	0.4	0.0	7.0
LnGrp Delay(d),s/veh	17.1	11.5	18.7	9.3	0.0	17.7
LnGrp LOS	В	В	В	Α		В
Approach Vol, veh/h	386			43	472	
Approach Delay, s/veh	16.7			11.1	17.7	
Approach LOS	В			В	В	
Timer	1	2	3	4	5	6
Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		32.0		28.0		32.0
Change Period (Y+Rc), s		4.5		4.5		4.5
Max Green Setting (Gmax), s		27.5		23.5		27.5
Max Q Clear Time (g_c+l1), s		16.9		11.7		16.4
Green Ext Time (p_c), s		5.5		1.8		5.7
<b>4</b> – <i>7</i>		J.U		1.0		J.1
Intersection Summary						
HCM 2010 Ctrl Delay			16.9			
HCM 2010 LOS			В			

	•	•	4	<b>†</b>	<b>↓</b>
Lane Group	EBL	EBR	NBL	NBT	SBT
Lane Group Flow (vph)	726	7	29	60	433
v/c Ratio	0.74	0.01	0.19	0.11	0.55
Control Delay	16.5	4.1	19.0	15.7	5.0
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	16.5	4.1	19.0	15.7	5.0
Queue Length 50th (m)	55.1	0.0	2.3	4.7	0.3
Queue Length 95th (m)	93.5	1.4	8.0	11.7	17.0
Internal Link Dist (m)	80.4			124.5	290.2
Turn Bay Length (m)			105.0		
Base Capacity (vph)	978	566	154	528	794
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.74	0.01	0.19	0.11	0.55
Intersection Summary					

				•	ı	
		•	7	T	<b>\</b>	*
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ň	7	Ţ	<b>†</b>	f)	
Traffic Volume (veh/h)	711	7	28	59	4	420
Future Volume (veh/h)	711	7	28	59	4	420
Number	7	14	5	2	6	16
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1881	1210	1827	1696	1845	1900
Adj Flow Rate, veh/h	726	7	29	60	4	429
Adj No. of Lanes	1	1	1	1	1	0
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	1	57	4	12	0	0
Cap, veh/h	970	557	162	523	4	480
Arrive On Green	0.54	0.54	0.31	0.31	0.31	0.31
Sat Flow, veh/h	1792	1029	933	1696	15	1556
Grp Volume(v), veh/h	726	7	29	60	0	433
Grp Sat Flow(s), veh/h/ln	1792	1029	933	1696	0	1571
Q Serve(g_s), s	18.7	0.2	1.8	1.5	0.0	15.8
Cycle Q Clear(g_c), s	18.7	0.2	17.6	1.5	0.0	15.8
Prop In Lane	1.00	1.00	1.00	1.5	0.0	0.99
Lane Grp Cap(c), veh/h	970	557	162	523	0	484
V/C Ratio(X)	0.75	0.01	0.18	0.11	0.00	0.89
Avail Cap(c_a), veh/h	970	557	162	523	0.00	484
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
						1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	
Uniform Delay (d), s/veh	10.6	6.3	28.2	14.9	0.0	19.8
Incr Delay (d2), s/veh	5.3	0.0	2.4	0.4	0.0	21.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	10.5	0.1	0.6	0.8	0.0	9.6
LnGrp Delay(d),s/veh	15.9	6.4	30.6	15.3	0.0	41.4
LnGrp LOS	В	А	С	В		D
Approach Vol, veh/h	733			89	433	
Approach Delay, s/veh	15.8			20.3	41.4	
Approach LOS	В			С	D	
Timer	1	2	3	4	5	6
Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		23.0		37.0		23.0
Change Period (Y+Rc), s		4.5		4.5		4.5
Max Green Setting (Gmax), s		18.5		32.5		18.5
Max Q Clear Time (q_c+l1), s		19.6		20.7		17.8
Green Ext Time (p_c), s		0.0		4.0		0.4
, , , , , , , , , , , , , , , , , , ,		0.0		4.0		0.4
Intersection Summary						
HCM 2010 Ctrl Delay			24.9			
HCM 2010 LOS			С			

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Lane Group	EBL	EBR	NBL	NBT	SBT
Lane Group Flow (vph)	358	28	13	35	479
v/c Ratio	0.53	0.05	0.07	0.07	0.51
Control Delay	17.6	5.4	10.2	9.6	4.5
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	17.6	5.4	10.2	9.6	4.5
Queue Length 50th (m)	29.1	0.0	0.8	2.0	4.8
Queue Length 95th (m)	50.6	3.9	3.4	6.1	19.5
Internal Link Dist (m)	80.4			124.5	290.2
Turn Bay Length (m)			105.0		
Base Capacity (vph)	674	541	200	527	934
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.53	0.05	0.07	0.07	0.51
Intersection Summary					

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Marrowal	EDI	EDD	NDI	NDT	CDT	CDD		
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	<b>\</b>	7	10	<b>↑</b>		270		
Traffic Volume (veh/h)	340	27	12	33	77	378		
Future Volume (veh/h)	340	27	12	33	77	378		
Number	7	14	5	2	6	16		
Initial Q (Qb), veh	1.00	1.00	1.00	0	0	1.00		
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00	1 00	1.00	1.00		
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00		
Adj Sat Flow, veh/h/ln	1792	1557	1203 13	1138 35	1748	1900		
Adj Flow Rate, veh/h	358 1	28 1	13	35 1	81	398		
Adj No. of Lanes Peak Hour Factor	0.95				1 0.95	0 0.95		
		0.95	0.95	0.95		27		
Percent Heavy Veh, %	6 669	22 510	58 244	67 521	27 118	580		
Cap, veh/h Arrive On Green	0.39	518 0.39	0.46	0.46	0.46	0.46		
	1707	1324	589	1138	258	1266		
Sat Flow, veh/h								
Grp Volume(v), veh/h	358	28	13	35	0	479		
Grp Sat Flow(s), veh/h/ln	1707	1324	589	1138	0	1524		
Q Serve(g_s), s	9.7	0.8	1.1	1.0	0.0	14.9		
Cycle Q Clear(g_c), s	9.7	0.8	16.0	1.0	0.0	14.9		
Prop In Lane Lane Grp Cap(c), veh/h	1.00 669	1.00 518	1.00 244	521	0	0.83 699		
	0.54		0.05		0.00	0.69		
V/C Ratio(X)	669	0.05 518	244	0.07 521		699		
Avail Cap(c_a), veh/h				1.00	1.00	1.00		
HCM Platoon Ratio	1.00 1.00	1.00 1.00	1.00 1.00	1.00	1.00 0.00	1.00		
Upstream Filter(I)	14.0	11.3	1.00	9.1	0.00	1.00		
Uniform Delay (d), s/veh Incr Delay (d2), s/veh	3.1	0.2	0.4	0.2	0.0	5.4		
Initial Q Delay(d3),s/veh	0.0	0.2	0.4	0.2	0.0	0.0		
%ile BackOfQ(50%),veh/ln	5.0	0.0	0.0	0.0	0.0	7.2		
LnGrp Delay(d),s/veh	17.1	11.5	19.6	9.3	0.0	18.3		
LnGrp LOS	17.1 B	11.5 B	19.0 B	9.3 A	0.0	16.3 B		
	386	ь	ь	48	479	D		
Approach Vol, veh/h								
Approach LOS	16.7 B			12.1 B	18.3 B			
Approach LOS	В			В	В			
Timer	1	2	3	4	5	6	7	8
Assigned Phs		2		4		6		
Phs Duration (G+Y+Rc), s		32.0		28.0		32.0		
Change Period (Y+Rc), s		4.5		4.5		4.5		
Max Green Setting (Gmax), s		27.5		23.5		27.5		
Max Q Clear Time (g_c+I1), s		18.0		11.7		16.9		
Green Ext Time (p_c), s		5.2		1.8		5.7		
Intersection Summary								
HCM 2010 Ctrl Delay			17.3					
HCM 2010 LOS			В					

	۶	•	4	†	<b>↓</b>
Lane Group	EBL	EBR	NBL	NBT	SBT
Lane Group Flow (vph)	726	7	35	60	435
v/c Ratio	0.74	0.01	0.27	0.11	0.55
Control Delay	16.5	4.1	22.1	15.7	5.1
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	16.5	4.1	22.1	15.7	5.1
Queue Length 50th (m)	55.1	0.0	2.9	4.7	0.5
Queue Length 95th (m)	93.5	1.4	9.7	11.7	17.3
Internal Link Dist (m)	80.4			124.5	290.2
Turn Bay Length (m)			105.0		
Base Capacity (vph)	978	566	131	528	793
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.74	0.01	0.27	0.11	0.55
Intersection Summary					

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
	EBL T					SDK
Lane Configurations Traffic Volume (veh/h)	<b>1</b> 711	<b>7</b>	<b>5</b>	<b>↑</b> 59	<b>1&gt;</b>	420
Future Volume (veh/h)	711	7	34	59	6	420
Number	711	14	5	2	6	16
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00	U	U	1.00
	1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj						1900
Adj Sat Flow, veh/h/ln	1881	1210	1570	1696	1838	
Adj Flow Rate, veh/h	726	7	35	60	6	429
Adj No. of Lanes	1	1	1	1	1	0
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	1	57	21	12	32	32
Cap, veh/h	970	557	154	523	7	476
Arrive On Green	0.54	0.54	0.31	0.31	0.31	0.31
Sat Flow, veh/h	1792	1029	801	1696	22	1544
Grp Volume(v), veh/h	726	7	35	60	0	435
Grp Sat Flow(s),veh/h/ln	1792	1029	801	1696	0	1565
Q Serve(g_s), s	18.7	0.2	2.5	1.5	0.0	16.0
Cycle Q Clear(g_c), s	18.7	0.2	18.5	1.5	0.0	16.0
Prop In Lane	1.00	1.00	1.00			0.99
Lane Grp Cap(c), veh/h	970	557	154	523	0	483
V/C Ratio(X)	0.75	0.01	0.23	0.11	0.00	0.90
Avail Cap(c_a), veh/h	970	557	154	523	0	483
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	10.6	6.3	28.8	14.9	0.0	19.9
Incr Delay (d2), s/veh	5.3	0.0	3.4	0.4	0.0	22.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	10.5	0.1	0.7	0.8	0.0	9.9
LnGrp Delay(d),s/veh	15.9	6.4	32.2	15.3	0.0	42.5
LnGrp LOS	В	Α	С	В		D
Approach Vol, veh/h	733			95	435	
Approach Delay, s/veh	15.8			21.5	42.5	
Approach LOS	В			С	D	
						,
Timer	1	2	3	4	5	6
Assigned Phs		2		4		6
Phs Duration (G+Y+Rc), s		23.0		37.0		23.0
Change Period (Y+Rc), s		4.5		4.5		4.5
Max Green Setting (Gmax), s		18.5		32.5		18.5
Max Q Clear Time (g_c+l1), s		20.5		20.7		18.0
Green Ext Time (p_c), s		0.0		4.0		0.3
Intersection Summary						
HCM 2010 Ctrl Delay			25.4			
HCM 2010 LOS			C			
110111 2010 200			0			

**AECOM** 

Appendix C

**Signal Warrant Outputs** 

Results	Sheet	Input Sheet Analys	Sis Sheet Propo	sed Collision GO TO Justification:
Intersection: E	Energy Drive/Bowmanville A	Avenue - 2024 BG Count D	ate: 2018	
Summary F	Results			
	Justification	Compliance	Signal Justified? YES NO	
1. Minimum Vehicular	A Total Volume	100 %		
Volume	B Crossing Volume	100 %		
2. Delay to Cross	A Main Road	58 %		
Traffic	B Crossing Road	100 %		
3. Combination	A Justificaton 1	100 %		
	B Justification 2	58 %		
4. 4-Hr Volume		90 %		
5. Collision Expe	erience	0 %		
6. Pedestrians	A Volume	Justification not met		
	A volume	Justilication not met		

Results	She	et	Input Sheet	Analysis	s Sheet	Propo	sed Collision	GO TO Justification:
Intersection: E	Energy [	Orive/Bowmanville A	venue - 2024 FT	Count Dat	e: 2018			
Summary I	Result	ts						
Justification		Compliano	Signal J	ustified?	]			
		Compilation		YES	NO			
1. Minimum Vehicular	А Т	otal Volume	100	%	~		-	
Volume	в с	rossing Volume	100	%				
2. Delay to Cross	A M	lain Road	60	%		✓		
Traffic	в с	rossing Road	100	%		Į.		
3. Combination	A Jı	ustificaton 1	100	%	П	V		
	B J	ustification 2	60	%				
4. 4-Hr Volume			87	%		~		
5. Collision Exp	erience		0	%		V		
6. Pedestrians	A V	olume	Justification not	met		-		

Results	She	eet	Input Sheet	Analysis	Sheet	Propo	sed Collision		GO TO Just
Intersection: B	Bowma	anville Avenue/Highw	ay 401 NRT - 2024 BG	Count Date	e: 2018				
Summary F	Resu	ilts							
Results Sheet  Intersection: Bowmanville Avenue/Highw  Summary Results  Justification  1. Minimum Vehicular Volume B Crossing Volume  2. Delay to Cross Traffic B Crossing Road  3. Combination A Justification 1 B Justification 2  4. 4-Hr Volume		Compliance	)	Signal Ji	ustified?				
	A	Total Volume	100	%		✓			
	В	Crossing Volume	93	%		•			
	Α	Main Road	98	%		V			
Cross Traffic B	В	Crossing Road	38	%					
3. Combination	A	Justificaton 1	93	%		V			
Intersection: Bowmanville Avenue/High Summary Results  Justification  1. Minimum Vehicular Volume B Crossing Volume  2. Delay to Cross Traffic B Crossing Road  3. Combination A Justification 1 B Justification 2	Justification 2	38	%		12				
4. 4-Hr Volume			100	%	~				
5. Collision Experience		0	%		•				
6. Pedestrians A Volume		Justification not m	net						

Results	She	eet	Input Sheet	Analysis	Sheet	Propo	sed Collision		GO TO Ju	GO TO Justification
Intersection: B	Bowma	anville Avenue/Highw	ay 401 NRT - 2024 FT	Count Date	e: 2018					
Summary F	Resu	ilts								
Results Sheet  Intersection: Bowmanville Avenue/Highw Summary Results  Justification  1. Minimum Vehicular Volume B Crossing Volume 2. Delay to Cross Traffic B Crossing Road 3. Combination A Justification 1 B Justification 2 4. 4-Hr Volume		Compliance	9	Signal J	ustified?					
	Α	Total Volume	100	%		⊽				
Volume	В	Crossing Volume	93	%		•				
	Α	Main Road	98	%		<b>V</b>				
Cross Traffic B C	Crossing Road	41	%							
3. Combination	Α	Justificaton 1	93	%		<b>V</b>				
	В	Justification 2	41	%		T.				
4. 4-Hr Volume			100	%	~					
5. Collision Experience		0	%		<b>~</b>					
6. Pedestrians A Volume		Justification not n								

Results	Sh	eet	Input Sheet	Analysis	Sheet	Propo	sed Collision		GO TO Jus	GO TO Justification:								
Intersection: E	Energ	y Drive/Highway 401	SRT - 2024 BG	Count Date	e: 2018													
Summary	Res	ults																
Vehicular Volume B Crossing  2. Delay to Cross Traffic B Crossing  3. Combination A Justificat B Justificat	ification	Compliance	)	Signal J														
				YES	NO													
	A	Total Volume	98	%		~												
Volume	В	Crossing Volume	100	%														
	A	Main Road	63	%		~												
Traffic	В	Crossing Road	100	%														
3. Combination	A	Justificaton 1	98	%		<b>V</b>												
	В	Justification 2	63	%														
4. 4-Hr Volume			92	%		~												
							-											
5. Collision Exp	erienc	ee	0	%		~												
						I.												
							1											
6. Pedestrians	Α	Volume	Justification not n	net		✓												
В	Delay	Justification not n	net															

Results	Sh	eet	Input Sheet	Analysis	Sheet	Propo	sed Collision		GO TO Just	GO TO Justification:								
Intersection: E	Energ	y Drive/Highway 401	SRT - 2024 FT	Count Date	e: 2018													
Summary I	Resi	ults																
1. Minimum Vehicular Volume B Crossir  2. Delay to Cross Traffic B Crossir  3. Combination A Justific B Justific  4. 4-Hr Volume	ification	Compliance	Compliance		ustified?													
				YES	NO													
	A	Total Volume	99	%		~												
Volume	e B Crossing A Main Roac B Crossing	Crossing Volume	100	%														
	Α	Main Road	63	%		~												
Traffic	В	Crossing Road	100	%														
3. Combination	Α	Justificaton 1	99	%		<b>V</b>												
	В	Justification 2	63	%														
4. 4-Hr Volume			92	%		~												
5. Collision Exp	erienc	e	0	%		~												
6. Pedestrians	А	Volume	Justification not m	net		<u>~</u>												
A	Delay	Justification not m	net		•													